



1 **Gravel threshold of motion: A state function of sediment**  
2 **transport disequilibrium?**

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7

8 **Abstract**

9 In most sediment transport models, a threshold variable dictates the shear stress at which non-  
10 negligible bedload transport begins. Previous work has demonstrated that nondimensional  
11 transport thresholds ( $\tau_c^*$ ) vary with many factors related not only to grain size and shape, but  
12 also with characteristics of the flow and surrounding grains. Both a conceptual model and  
13 flume experiments suggest that  $\tau_c^*$  should evolve as a function of local entrainment and  
14 deposition. Net entrainment preferentially removes more mobile particles while leaving  
15 behind more stable grains, gradually increasing  $\tau_c^*$  and reducing transport rates. Net  
16 deposition tends to fill in topographic lows, progressively leading to less stable distributions  
17 of surface grains, decreasing  $\tau_c^*$  and increasing transport rates. A new model is proposed for  
18 the temporal evolution of  $\tau_c^*$  as a power-law function of net erosion or deposition, which  
19 shares some similarities with the Exner equation. Model parameters are calibrated based on  
20 flume experiments that explore transport disequilibrium. The  $\tau_c^*$ -evolution equation is then  
21 incorporated into a simple morphodynamic model. The evolution of  $\tau_c^*$  acts as negative  
22 feedbacks on morphologic change, while also allowing reaches to equilibrate to sediment  
23 supply at different slopes. Finally,  $\tau_c^*$  is interpreted to be an important but nonunique state  
24 variable for channel morphology, in a manner consistent with the role that state variables such  
25 as temperature play in describing the evolution of thermodynamic systems.

26



## 27 1 Motivation

28 Despite over a century of quantitative study (Gilbert, 1914), it often remains  
29 challenging to predict coarse bedload transport rates to much better than an order of  
30 magnitude because of the complexity of grain interactions with the flow and the surrounding  
31 grains. Predictive models for complex systems often derive utility from their simplicity, as is  
32 the case with the widely used Meyer-Peter and Müller (1948) transport equation, as modified  
33 by Wong and Parker (2006):

$$34 \quad q_s^* = 3.97(\tau^* - \tau_c^*)^{1.5} \quad \text{for} \quad \tau^* \geq \tau_c^* \quad (1)$$

35 where  $q_s^*$  is a nondimensional sediment transport rate per unit width,  $\tau^*$  is a nondimensional  
36 shear stress imparted by the fluid on the channel bed (a Shields stress), and  $\tau_c^*$  is the  
37 nondimensional threshold stress at which grains begin to move (a critical Shields stress).  
38 Variables are nondimensionalized as followed:

$$39 \quad q_s^* = \frac{q_s}{D \sqrt{\left(\frac{\rho_s}{\rho} - 1\right) g D}} \quad (2)$$

$$40 \quad \tau^* = \frac{\tau}{(\rho_s - \rho) g D} \quad (3)$$

41 where  $q_s$  is volume sediment transport rate per unit width ( $\text{m}^2/\text{s}$ ),  $D$  is grain diameter (m),  
42  $\rho_s$  is sediment density ( $\text{m}^3/\text{kg}$ ),  $\rho$  is water density ( $\text{m}^3/\text{kg}$ ),  $g$  is gravitational acceleration  
43 ( $\text{m}/\text{s}^2$ ), and  $\tau$  is shear stress (Pa). In principle, these nondimensionalizations should account  
44 for differences in grain size, fluid and sediment density and gravity, allowing meaningful  
45 comparisons of transport and stress across different conditions. For a given grain diameter  
46 (and ~constant  $\rho_s$ ,  $\rho$  and  $g$  assumed for terrestrial landscapes), the simplicity of Eq. (1) is  
47 that it predicts transport rate using just two variables,  $\tau^*$  (a function of flow strength) and  $\tau_c^*$   
48 (a function of many variables). In practice,  $\tau_c^*$  is often back-calculated from a particular  
49 sediment transport model. For example, the Meyer-Peter and Muller (1948)  $\tau^*$  and  $q_s^*$  give  
50 best-fit  $\tau_c^* = 0.0495$  for Eq. (1) (Wong and Parker, 2006). In practice,  $\tau_c^*$  can essentially be an  
51 empirical fitting parameter for a given transport model (e.g., Wong and Parker, 2006;  
52 Buffington and Montgomery, 1997).



53           Thresholds of motion for gravel often span an order of magnitude or more (Fig. 1; see  
54 caption for data sources). Variability in  $\tau_c^*$  greatly influences bedload flux predictions in  
55 mountain rivers because transport occurs close to thresholds conditions, even during large  
56 floods (Phillips et al., 2013; Parker et al., 1982; Parker and Klingeman, 1982). Previous work  
57 demonstrates a great many factors that can collectively cause  $\tau_c^*$  scatter; for example, slope  
58 can empirically explain 34% of the variability shown in Fig. 1 data (e.g., Buffington and  
59 Montgomery, 1997; Kirchner et al., 1990; Lamb et al., 2008). Although interrelated,  $\tau_c^*$   
60 influences can generally be classified as grain controls, “bed state” controls, and flow  
61 controls. In addition to diameter and density, grain controls include shape and angularity (e.g.,  
62 Prancevic and Lamb, 2015; Gogus and Defne, 2005). Bed surface controls include the grain  
63 size distribution (GSD) of the surrounding bed, the degree of overlap and interlocking among  
64 grains, the protrusion of grains into the flow, the degree of coarse grain clustering, the bed  
65 roughness, and the reach slope (e.g., Parker, 1990; Wilcock and Crowe, 2003; Sanguinito and  
66 Johnson, 2012; Buscombe and Conley, 2012; Mao, 2012; Kirchner et al., 1990; Strom and  
67 Papanicolaou, 2009; Marquis and Roy, 2012; Powell and Ashworth, 1995; Richards and  
68 Clifford, 1991). Flow characteristics influencing  $\tau_c^*$  include particle Reynolds number, flow  
69 depth relative to grain size, the intensity of turbulence, the history of prior flow both above  
70 and below transport thresholds, and the partitioning of stress into form drag and skin friction  
71 (e.g., Shvidchenko and Pender, 2000; Ockelford and Haynes, 2013; Schneider et al., 2015a).  
72 However, most flow-dependent controls are not independent of the bed surface controls. For  
73 example, flow depths, turbulence and form drag depend on slope and bed roughness, while  
74 the stress history influences  $\tau_c^*$  by changing grain interlocking and surface roughness.

75           In addition, recent work suggests that the amount of sediment supplied from upstream  
76 affects  $\tau_c^*$ , with higher rates of upstream supply corresponding to more mobile sediment and  
77 lower  $\tau_c^*$  (Recking, 2012; Bunte et al., 2013). Sensitivity of  $\tau_c^*$  to sediment flux is not  
78 obviously classifiable as either a flow or a bed state control. The idea that transport rate  
79 influences  $\tau_c^*$  is intriguing because, by definition,  $\tau_c^*$  influences transport rate (Eq. 1). This  
80 feedback is the focus of the present analysis. A constant  $\tau_c^*$  is commonly assumed for gravel  
81 transport calculations, perhaps for several reasons. First, the traditional Shields diagram  
82 indicates that  $\tau_c^*$  is rather insensitive to particle Reynolds number once flow becomes



83 hydraulically rough around grains (Buffington, 1999). Second, a belief that  $\tau_c^*$  is  
84 fundamentally a material property of a grain rather than a bed state control also remains  
85 somewhat ingrained. Third, because the best estimate of a given variable is usually its  
86 average, there is a tendency to attribute variability to measurement noise and uncertainty,  
87 even when that variability may be real, its causes understandable, and its influence potentially  
88 important to system dynamics (Jerolmack, 2011; Buffington and Montgomery, 1997).

89 Feedback between channel morphology and bedload transport defines  
90 morphodynamics. The Exner equation of sediment mass balance quantifies how transport  
91 changes correspond to topographic changes (Paola and Voller, 2005):

$$92 \quad \frac{\partial z}{\partial t} = - \left( \frac{1}{1 - \lambda_p} \right) \frac{\partial q_s}{\partial x} \quad (4)$$

93 where  $z$  is bed elevation (vertical position)  $x$  is horizontal position,  $t$  is time, and  $\lambda_p$  is bed  
94 porosity. Sediment flux and bed elevation are functions of both position and time. In this  
95 morphodynamic equation (presented for simplicity without an uplift or subsidence term),  
96 topographic equilibrium ( $\partial z / \partial t = 0$ ) is attained when the sediment flux into a reach equals the  
97 sediment flux out ( $\partial q_s / \partial x = 0$ ). Channel morphology has long been recognized to influence  
98 sediment transport. Of particular relevance to the present work, Stark and Stark (2001)  
99 proposed an insightful landscape evolution model with a variable called *channelization* that is  
100 defined as representing “the ease with which sediment can flux through a channel reach”  
101 (Stark and Stark, 2001). Conceptually, channelization characterizes how changes in reach  
102 morphology influence local transport rate. However, channelization is an abstract unitless  
103 number that does not correspond physically to any measureable aspects of morphology. A  
104 fundamental feedback is imposed in the model: channelization evolves through time as a  
105 function of both sediment flux and of itself, resulting in a differential equation. The  
106 combination of local slope and channelization tend to asymptote towards values such that  
107  $\partial q_s / \partial x = 0$ , i.e. transport equilibrium. For a given upstream sediment supply rate, a modeled  
108 reach can evolve to equilibrium at different slopes (for different corresponding values of  
109 channelization) because both slope and channelization affect transport rate.

110 Interestingly, the above definition of channelization could also be applied to  $\tau_c^*$ .  
111 Because of its control on transport rates, changes in  $\tau_c^*$  should influence channel



112 morphodynamics, both over human timescales (e.g., in response to natural and anthropogenic  
113 perturbations such as landslides, floods, post-wildfire erosion, land use, changing climate) and  
114 longer timescales (landscape evolution). The overall goal of the present work is to elucidate  
115 possible feedbacks among thresholds of motion, changes in transport rate, and the  
116 morphological evolution of channels. A motivating hypothesis is that variability in gravel  $\tau_c^*$   
117 is both physically meaningful and reflects changes in  $\tau_c^*$  as a predictable function of sediment  
118 flux. Another hypothesis is that the implicit effects of multiple processes on  $\tau_c^*$  can  
119 collectively be accounted for in terms of sediment flux dependence. Because changes in  
120 alluvial channel morphology are strongly coupled with sediment flux (Eq. 4), I also  
121 hypothesize that the evolution of  $\tau_c^*$  can implicitly model effects of evolving channel  
122 morphology.

123 The paper is organized as follows. First, a conceptual model for why  $\tau_c^*$  should  
124 depend on sediment flux is presented, followed by equations. Next, flume experiments on  
125 mixed grain size transport are used to empirically calibrate model parameters. The  
126 experimental data are consistent with the Wilcock and Crowe (2003) hiding function that  
127 predicts transport rates for grain size mixtures. A simple morphodynamic model is then used  
128 to evaluate how evolving  $\tau_c^*$  influences timescales of channel profile evolution. Finally, in  
129 the discussion section  $\tau_c^*$  is argued to be one of many “state variables” that can describe how  
130 channels evolve in response to external forcing and internal feedbacks. Comparisons are  
131 made to state variables in thermodynamics.

132

## 133 **2 Models and Methods**

### 134 **2.1 Conceptual framework for $\tau_c^*$ evolution**

135 For bed surface grains of a given size class,  $\tau_c^*$  actually represents the stress at which  
136 only the most mobile of those grains become entrained. Individual grains will each have a  
137 different threshold based on the pocket geometry and near-bed flow velocity at its particular  
138 location, resulting in  $\tau_c^*$  probability distributions when considering all similarly sized grains  
139 (Kirchner et al., 1990; Buffington et al., 1992). Gravel flux increases with discharge primarily



140 because thresholds are gradually exceeded for increasing proportions of surface grains of a  
141 given size. When the sediment flux entering a reach— $q_{\text{sin}}$ —is balanced by the flux exiting—  
142  $q_{\text{out}}$ —it is assumed that the  $\tau_c^*$  probability distribution does not change over time, and that  
143  $\tau_c^*$  remains constant.

144 In the case of a channel reach undergoing net erosion ( $q_{\text{out}} > q_{\text{sin}}$ , so  $\partial q_s / \partial x > 0$ ), the  
145 most mobile grains would preferentially be entrained first. Progressive erosion will entrain  
146 grains from increasingly more stable positions on the bed, gradually increasing  $\tau_c^*$ .  
147 Conversely, during net deposition, grains will tend to preferentially deposit in more stable bed  
148 positions such as local topographic lows. Continued deposition would lead to grains being  
149 deposited in progressively less stable positions, gradually decreasing  $\tau_c^*$ . These changes  
150 describe negative transport feedbacks: net erosion progressively reduces rates of erosion by  
151 making grains harder to entrain, while net deposition progressively makes grains more mobile  
152 and less likely to be deposited.

153 The amount by which  $\tau_c^*$  changes should also depend on the current state of the bed  
154 surface. If bed surface grains are already loosely packed and highly mobile, additional  
155 deposition would cause little additional decrease in  $\tau_c^*$ . On the other hand, initial deposition  
156 onto a stable bed would likely cause bigger reductions in  $\tau_c^*$  than subsequent deposition.  
157 Thus, the change in  $\tau_c^*$  should also be a function of  $\tau_c^*$ .

158 Two additional points need to be made. First, natural sediment is a mixture of sizes. It  
159 is common to assume that a single representative grain size, such as the median ( $D_{50}$ ),  
160 adequately describes transport of the whole mixture. Second, “thresholds” can represent  
161 different things in different models. In Eq. (1),  $\tau^* = \tau_c^*$  represents a modeled transport rate of  
162 zero. In other models designed to predict measurable transport rates at very low shear stresses,  
163 a non-threshold “reference” stress is instead defined as the Shields stress at which  $q_s^*$  has a  
164 very low but specific nonzero value (Parker, 1990; Wilcock and Crowe, 2003). For many  
165 applications the practical difference between threshold and reference stresses are negligible  
166 (Buffington and Montgomery, 1997), and in the present work they are largely used  
167 interchangeably.



168 **2.2  $\tau_c^*$ -evolution model equations**

169 Because longitudinal coordinate  $x$  increases downstream, net deposition in a reach is  
 170 indicated by  $\partial q_s / \partial x < 0$  and local erosion by  $\partial q_s / \partial x > 0$ . The following relations are  
 171 proposed:

$$172 \quad \frac{\partial \tau_c^*}{\partial t} = \begin{cases} kB \left( \frac{\partial q_s}{\partial x} \right)^{\kappa_{ent}} & \text{if } \partial q_s / \partial x > 0 \\ -k(1-B) \left( \frac{\partial q_s}{\partial x} \right)^{\kappa_{dep}} & \text{if } \partial q_s / \partial x < 0 \end{cases} \quad (5)$$

$$173 \quad B = \frac{\tau_{c\max}^* - \tau_c^*}{\tau_{c\max}^* - \tau_{c\min}^*} \quad (6)$$

174 where  $\kappa_{ent}$  and  $\kappa_{dep}$  are dimensionless exponents corresponding to entrainment and  
 175 deposition, respectively, and  $k$  is a scaling factor. These three parameters will be empirically  
 176 fit to experiments.  $\tau_{c\min}^*$  and  $\tau_{c\max}^*$  represent bounds on how low or high  $\tau_c^*$  can plausibly  
 177 evolve (assumed to be 0.02 and 0.35 respectively). Eq. (5) predicts that  $\tau_c^*$  incrementally  
 178 decreases when net deposition occurs, and incrementally increases during net erosion in a  
 179 reach. “Feedback factor”  $B$  makes Eq. (5) a differential equation; it scales the incremental  
 180 change in  $\tau_c^*$  such that deposition on an already “loose” bed ( $\tau_c^*$  close to  $\tau_{c\min}^*$ ) would be  
 181 minimally decrease  $\tau_c^*$ , but erosion would cause a larger incremental  $\tau_c^*$  increase.  
 182 Conversely, if  $\tau_c^*$  were already high (close to  $\tau_{c\max}^*$ ), then erosion would cause a much  
 183 smaller  $\tau_c^*$  change than deposition. Finally, we note that representing  $\partial \tau_c^* / \partial t$  as a function of  
 184  $\partial q_s / \partial x$  is broadly analogous in form to Exner (Eq. 4).

185 A limitation of Eq. (5) is that, for dimensionally consistency, the units of  $k$  vary with  
 186  $\kappa_{ent}$  and  $\kappa_{dep}$ . An improved equation replaces spatial changes in flux with spatial changes in  
 187 the thickness of deposited or eroded sediment:

$$188 \quad \frac{\partial \tau_c^*}{\partial t} = \begin{cases} kA_r B \left( \frac{\partial \theta_s}{\partial x} \right)^{\kappa_{ent}} & \text{if } \partial q_s / \partial x > 0 \\ -kA_r (1-B) \left( \frac{\partial \theta_s}{\partial x} \right)^{\kappa_{dep}} & \text{if } \partial q_s / \partial x < 0 \end{cases} \quad (7)$$



189  $A_r$  is an optional dimensionless armoring parameter, described further below.  $\theta_s$  is the  
190 thickness of sediment deposited or eroded at a given location; it has dimensions of length.  
191  $\partial\theta_s/\partial x$  is a dimensionless ratio representing spatial changes in erosion and deposition. In this  
192 case,  $k$  has dimensions  $1/t$  and scales how quickly  $\tau_c^*$  evolves.  $\theta_s$  can be calculated by  
193 integrating Eq. (4) over time interval  $t_1$  to  $t_2$ :

$$194 \quad \theta_s(t_2, x) = z(t_2, x) - z(t_1, x) = \frac{1}{1 - \lambda_p} \int_{t_1}^{t_2} \frac{\partial q_s(t, x)}{\partial x} dt \quad (8)$$

195 (recall that  $\int_a^b (\partial f(s, t)/\partial t) dt = f(s, b) - f(s, a)$  for a generic function  $f$ ). Using discrete flume  
196 data,  $\theta_s$  is calculated over a measurement interval  $\Delta t$  as  $(1 - \lambda_p)^{-1} (\overline{q_{sout}} - \overline{q_{sin}}) \Delta t / \Delta x$ , where  
197  $\Delta x$  is the length of the flume and the sediment flux terms are averaged over  $\Delta t$ .

198 While  $A_r$  is set to 1 for many calculations below, the parameter is included in order to  
199 explore whether other metrics of relative surface grain size variability or bed roughness  
200 improve predictions. For example, the rate at which  $\tau_c^*$  changes might depend on grain size  
201 relative to bed surface roughness ( $\sigma$ ), i.e.  $A_r = D_{50}/\sigma$ . Setting  $A_r = D_{50}/\sigma$  suggests that,  
202 relative to topographic lows and highs, large range cause bigger  $\tau_c^*$  changes than small grains.

### 203 2.3 Experimental design

204 The flume experiments used to calibrate  $k$ ,  $\kappa_{ent}$  and  $\kappa_{dep}$  explore how fine gravel  
205 pulses influence the morphodynamics of step-pool like channels. Johnson et al. (2015)  
206 provide details of the experimental conditions and how they scale to natural conditions, and  
207 so the summary here is brief. Four experiments were conducted in a small flume 4 m long and  
208 10 cm wide. Experiments 1 and 4 were done at 8% initial slope, and 2 and 3 at 12% initial  
209 slope. Water discharge was held constant throughout to better isolate the influence of  
210 sediment supply changes on transport. Sediment transported out of the flume over different  
211 time intervals was caught in a downstream basket, sieved and weighed. Sediment was painted  
212 different colors based on five size classes with median diameters 2.4, 4.5, 8.0, 15.4, and 27.2  
213 mm. Surface GSDs were measured using image analysis of colored bed surface grains during  
214 the experiments. Bed topography was measured using a triangulating laser, and bed roughness  
215 ( $\sigma$ ) was calculated from longitudinal topographic swaths as the standard deviation of





216 detrended bed elevations. Water surface elevations were measured using an ultrasonic  
217 distance sensor, and water depths were calculated by subtracting bed elevations. Total shear  
218 stress ( $\tau$ ) was calculated assuming steady uniform flow when spatially averaged over the  
219 flume:

$$220 \quad \tau = \rho ghS \quad (9)$$

221 where  $h$  is water depth corrected for sidewall effects following the method of Wong and  
222 Parker (2006), and  $S$  is water surface slope.

223 The experiments started with mixed-size sediment screeded flat. Initially, all surface  
224 sizes were observed to be mobile (and therefore above thresholds of grain motion). At the  
225 beginning no sediment was fed into the upstream end ( $q_{\text{sin}}=0$ ), and the bed responded by  
226 coarsening, roughening and some net erosion. The bed gradually stabilized and transport rates  
227 dropped to be very low. After this initial bed stabilization, a step-function pulse of the finest  
228 gravel size ( $D_{50}=2.4$  mm) was fed into the flume at 1000 g/min, representing an idealization  
229 of a landslide, debris flow, post-wildfire erosion, anthropogenic gravel augmentation, or  
230 similar process that would suddenly supply a prolonged pulse of sediment finer than the  
231 existing bed surface. Initially some deposition occurred on the bed, but the channel adjusted  
232 rapidly, by both entraining coarser bed surface grains and transporting most of the finer  
233 supplied gravel, so that the outlet transport rate ( $q_{\text{out}}$ ) approximately matched  $q_{\text{sin}}$ . After the  
234 sediment supply pulse  $q_{\text{sin}}$  was again dropped to zero, and the bed gradually restabilized,  
235 asymptotically approaching though not quite attaining equilibrium transport ( $q_{\text{out}} \approx 0$ ) during  
236 the remainder of each experiment.

#### 237 **2.4 The Wilcock and Crowe (2003) transport model**

238 As described above,  $\tau_c^*$  is often calculated by fitting a sediment transport model to  
239 data. Experimental shear stresses and GSDs provide all of the variables needed to evaluate the  
240 “Surface-based Transport Model for Mixed-Size Sediment” of Wilcock and Crowe (2003),  
241 abbreviated as W&CM. A goal is to evaluate how well the W&CM can predict transport in  
242 the present experiments, which used steeper reach slopes and lower water depths relative to  
243 GSDs compared to experiments of Wilcock and Crowe (2003). In addition, disequilibrium  
244 transport was intentionally quantified during supply perturbations that caused net deposition



245 or erosion. In contrast, the experiments Wilcock and Crowe (2003) used to calibrate their  
 246 model intentionally reflected steady-state transport.

247 A central variable in the W&CM is  $\tau_{rs50}$ , the reference stress for the median *surface*  
 248 grain size ( $D_{s50}$ ). The nondimensional equivalent is  $\tau_{rs50}^*$  (Eq. 3). Rather than being an  
 249 actual threshold (i.e.,  $\tau_c^*$ ),  $\tau_{rs50}^*$  corresponds to a very low transport rate of  $W_i^*=0.002$ .  $W_i^*$  is  
 250 a nondimensional bedload transport rate for grain size class  $i$ ,

$$251 \quad W_i^* = \left( \frac{\rho_s}{\rho} - 1 \right) \frac{g q_{bi}}{F_i u_\tau^3} \quad (10)$$

252 where  $q_{bi}$  is the volumetric transport rate per unit channel width of grains of size  $i$ ,  $F_i$  is the  
 253 fraction of size  $i$  on the bed surface, and  $u_\tau$  is shear velocity ( $u_\tau = \sqrt{\tau/\rho}$ ). Wilcock and  
 254 Crowe (2003) presented an empirical relationship between transport and shear stress:

$$255 \quad W_i^* = \begin{cases} 0.002 \left( \frac{\tau}{\tau_{ri}} \right)^{7.5} & \text{for } \tau/\tau_{ri} < 1.35 \\ 14 \left[ 1 - \frac{0.894}{\left( \frac{\tau}{\tau_{ri}} \right)^{0.5}} \right]^{4.5} & \text{for } \tau/\tau_{ri} \geq 1.35 \end{cases} \quad (11)$$

256 where  $\tau_{ri}$  is a dimensional reference stress for size class  $i$ , with dimensionless equivalent  $\tau_{ri}^*$   
 257 (Eq. 3). A “hiding function” determines how nondimensional reference stresses vary with  
 258 grain size:

$$259 \quad \frac{\tau_{ri}^*}{\tau_{rs50}^*} = \left( \frac{D_i}{D_{s50}} \right)^{b-1} \quad (12)$$

260 Hiding function exponent  $b$  is calculated as

$$261 \quad b = \frac{0.67}{1 + e^{(1.5 - D_i/D_{s50})}} \quad (13)$$

262 Note that the W&CM  $b$  relation was slightly modified by replacing  $D_{sm}$  (the geometric mean  
 263 surface diameter) with  $D_{s50}$ , to more simply use just one measure of the central tendency of



264 the surface GSD. The additional dependence of  $\tau_{rs50}^*$  on surface sand fractions in this model  
265 will be addressed below.

266

## 267 **3 Results**

### 268 **3.1 Experimental data and best-fit $\tau_{rs50}^*$ , hiding functions**

269 Transport rates, surface grain size, and reach slopes evolved during the experiments as  
270 the flume beds initially stabilized, then responded to the pulse of sediment supply, and finally  
271 restabilized (Fig. 2). Johnson et al. (2015) explained in detail how the addition of gravels  
272 finer than the stabilized bed surface ultimately caused further surface coarsening. Relevant to  
273 the present analysis, transport rates in and out of the flume are not always balanced (Fig. 2a),  
274 although transport evolves towards this equilibrium condition.

275 The experimental data are used to determine  $W_i^*$  (Eq. 10) and then  $\tau_{ri}^*$  for each of the  
276 five size classes (Eq. 11). Best-fit  $\tau_{rs50}^*$  is then calculated in two ways. In the first approach,  $b$   
277 is simply calculated using Eq. (13). Nonlinear multiple regression (in Matlab) is then used  
278 for best-fit parameter estimation of  $\tau_{rs50}^*$  and 95% confidence intervals for each experimental  
279 time step (Fig. 3, “W&C fit”). A key point to emphasize is that experimental grain size  
280 changes do not explain the temporal evolution of best-fit  $\tau_{rs50}^*$ , because in this analysis the  
281 W&CM should already account for grain size changes in terms of  $D_i/D_{s50}$  (Eq. 12, 13). Also  
282 note that a form drag correction was not done; transport was calculated in terms of total stress  
283 (Eq. 9; section 5.5).

284 While hiding function exponent  $b$  varies with relative grain size in the W&CM (Eq.  
285 13), other proposed hiding functions have usually found (or assumed) that a single  $b$  value  
286 applies to different grain sizes, at least for a given set of flow and surface conditions (Parker,  
287 1990; Buscombe and Conley, 2012). The second approach explores whether  $\tau_{rs50}^*$  estimates  
288 are sensitive to details of the hiding function exponent. Rather than using Eq. (13),  $b$  is  
289 empirically fit as single values (for all grain size classes), but with different fits to  $b$  for each  
290 separate time step. Nonlinear multiple regression was used to estimate both  $b$  and  $\tau_{rs50}^*$  in Eq.  
291 (12) (Fig. 3). Although the 95% confidence intervals tend to be larger because both  $b$  and



292  $\tau_{rs50}^*$  were estimated rather than just  $\tau_{rs50}^*$ , the temporal evolution of experimental  $\tau_{rs50}^*$  is  
293 comparable for the two different approaches (Fig. 3).

294 Interestingly, these experiments are consistent with the particular grain size-dependent  
295  $b$  function proposed by Wilcock and Crowe (2003) (Eq. 13). Fig. 4 shows data points  
296 determined using the  $b$  and  $\tau_{rs50}^*$  regressions, *not* using Eq. (13). In spite of substantial scatter  
297 there is a slope break which corresponds to a change in  $b$  for surface grains smaller and larger  
298 than the median.

### 299 3.2 Calibration of $\tau_c^*$ -evolution model parameters

300 Fig. 5 compares the experiment and W&CM-based calculation of  $\tau_{rs50}^*$  to several  
301 predictions of these trends. First, a sand fraction dependence is explored. A unique aspect of  
302 the W&CM not described above is that changes in surface sand fraction could cause temporal  
303 evolution of  $\tau_{rs50}^*$ . In particular, Wilcock and Crowe (2003) proposed that  $\tau_{rs50}^*$  varied  
304 systematically from 0.036 with no surface sand to 0.021 with abundant surface sand:

$$305 \tau_{rs50}^* = c_1 + c_2 e^{-c_3 F_s} \quad (14)$$

306 where  $F_s$  is the fraction of sand on the bed surface, and constants  $c_1$ ,  $c_2$  and  $c_3$  were  
307 empirically found by them to be 0.021, 0.015 and 20 respectively (for simplicity, the  
308 geometric mean reference stress was again replaced in their original equation with  $\tau_{rs50}^*$ ).  
309 Subsequent work has shown that thresholds of motion can similarly be reduced by grains  
310 substantially smaller than the bed surface but larger than sand (Venditti et al., 2010; Sklar et  
311 al., 2009; Johnson et al., 2015), suggesting that the “sand fraction” effect could also be  
312 modeled for grains larger than 2 mm.

313 In the present experiments, the surface fraction of actual sand (<2 mm) was very  
314 small. However, because grains larger than sand but smaller than the average bed surface  
315 may also enhance mobility, we evaluate whether Eq. (14) can explain the experimentally-  
316 constrained variations in  $\tau_{rs50}^*$ . Using the surface fraction of grains < 2.8 mm (representing the  
317 smallest grain size class in the experiments), a nonlinear multiple regression of Eq. (14) to all  
318 four experiments together yielded a poor although statistically significant fit to the data



319 ( $R^2=0.13$ ;  $p=3\times 10^{-5}$ ;  $c_1=0.097\pm 0.04$ ,  $c_2=0.103\pm 0.11$ , and  $c_3=5.6\pm 11$ ), confirming that surface  
320 grain size changes alone cannot explain observed  $\tau_{rs50}^*$  patterns (Fig. 5, “Sand fraction”).

321 My proposed  $\tau_c^*$ -evolution models that instead depend on transport disequilibrium  
322 provide much better fits to experimentally-constrained  $\tau_{rs50}^*$ . Fig. 5 shows model fits using  
323 Eq. (7) with  $A_r=1$ , both for a single set of model parameters that provide the best fit to all four  
324 experiments together (“collective best fit”,  $k=0.17$ ,  $\kappa_{dep}=0.20$ ,  $\kappa_{ent}=0.40$ ), and also best fits  
325 for each of the four experiments separately. The best-fit overall model has  $R^2=0.69$ ,  
326 suggesting statistically that effects of supply and transport disequilibrium can explain over 2/3  
327 of the variability in  $\tau_{rs50}^*$  (Table 1). Note that  $\tau_c^*$  and  $\tau_{rs50}^*$  are assumed to be interchangeable  
328 for these model fits. The procedure used for estimating best-fit model parameters for Eq. (5)  
329 and (7) was different from that described above. The fact that these models are differential  
330 equations prevented the use of nonlinear multiple regressions. Instead, a brute-force approach  
331 was done of incrementally stepping through a wide range of parameter space of  $k$ ,  $\kappa_{dep}$  and  
332  $\kappa_{ent}$ , and finding the combination of parameters that gave the smallest RMSD. The  
333 calculations of  $\tau_{rs50}^*$  through time were started at  $\tau_{rs50}^*=0.036$  at  $t=0$ , which is consistent with  
334 the experiments, and also is the  $\tau_{rs50}^*$  proposed by Wilcock and Crowe (2003) in the absence  
335 of sand dependence.

336 Interestingly, model fits using  $A_r = D_{s50}/\sigma$  are not substantially different from  $A_r=1$ ,  
337 and  $R^2=0.69$  is the same (Fig. 5). Table 1 includes three additional regressions not shown on  
338 Fig. 5 because the fits overlap almost completely with those already shown.  
339  $A_r = D_{s50}/D_{s84}$  was tried, because the inverse ratio  $D_{s84}/D_{s50}$  represents the degree of  
340 armoring in the analysis of  $\tau_c^*$  supply dependence by Recking (2012).  
341  $A_r = 2D_{s50}/(D_{s84} - D_{s16})$  was also tried as a measure of the normalized GSD width. Finally,  
342 parameters were estimated for dimensional  $\partial q_s/\partial x$  (Eq. 5). Because these variants do not  
343 substantially improve  $\tau_c^*$  model fits, we use the simplest dimensionally consistent model (Eq.  
344 7 with  $A_r=1$ ) in the analysis below.

345



### 346 3.3 Influence of $\tau_c^*$ evolution on morphodynamics

#### 347 3.3.1 Morphodynamic model development

348 Next, an idealized morphodynamic model demonstrates how the proposed  $\tau_c^*$  relations  
349 influence the evolution of channel reach profiles, focusing on reach slopes and timescales of  
350 adjustment. Because the modeling goal is to isolate and understand effects of evolving  $\tau_c^*$ ,  
351 the underlying model is arguably the simplest reasonable representation of morphodynamic  
352 feedback. Inspired by G. Parker's Morphodynamics e-book, the model describes a channel  
353 reach in which slope evolves through aggradation and degradation. The downstream  
354 boundary elevation is fixed (constant base level). Sediment transport and bed elevation are  
355 modeled using Eq. (1) and (4) with a single grain diameter ( $D$ ). Unit water discharge  $q_w$  is  
356 similarly held constant for simplicity. Upstream sediment supply ( $q_{seed}$ ) is imposed, and is  
357 varied in simulations below to drive channels to new steady states. Relationships among flow  
358 depth, depth-averaged velocity and discharge are imposed by assuming that hydraulic  
359 roughness remains constant, parameterized through a Darcy-Weisbach hydraulic friction  
360 coefficient:

$$361 \quad f = \frac{8gq_w S}{U^3} \quad (15)$$

362 For a given discharge this allows both  $U$  and  $h$  to be determined:

$$363 \quad U = \frac{q_w}{h} \quad (16)$$

$$364 \quad h = q_w^{2/3} \left( \frac{f}{8gS} \right)^{1/3} \quad (17)$$

365 Although not presented, simulations were also done in which the relation between  $U$  and  $h$   
366 was determined by instead holding Froude number ( $Fr = U/\sqrt{gh}$ ) constant (Grant, 1997).  
367 While  $f$  changes systematically with slope in this scenario, resulting trends in reach slope  
368 adjustment and response timescales are substantively the same as shown below for constant  $f$ ,  
369 suggesting little sensitivity to the underlying hydraulic closure assumptions.



370 Two model variations are compared: in the “Exner-only” morphodynamic model,  $\tau_c^*$   
371 is held constant. In the “Exner+ $\tau_c^*$ ” variant,  $\tau_c^*$  evolves through time following Eq. (7). At  
372 equilibrium, channel slope can be predicted by combining Eq. (1), (2), (9) and (17):

$$373 \quad S_{eq} = \frac{2.83}{q_w} \left( \frac{g}{f} \right)^{1/2} D^{3/2} \left( \frac{\rho_s}{\rho} - 1 \right)^{3/2} \left[ \left( \frac{q_s^*}{3.97} \right)^{2/3} + \tau_c^* \right]^{3/2} \quad (18)$$

374 For a given discharge, Eq. (18) indicates that both sediment supply and the threshold of  
375 motion influence steady-state morphology (slope).

376 Away from equilibrium, rates of bed elevation change along a river profile should  
377 depend not only on the sediment flux at a given channel cross section, but also on the average  
378 velocity at which grains move downstream. This control has occasionally been ignored in  
379 previous models of profile evolution. In my model, it is crudely incorporated by assuming that  
380 average bedload velocity is a consistent fraction of water velocity, broadly consistent with  
381 previous findings that bedload velocities are proportional to shear velocity (e.g., Martin et al.,  
382 2012). The modeling timestep is set to be equal to the time it takes sediment to move from  
383 one model node (bed location) to the next, and is adjusted during simulations. While this  
384 approach makes the temporal evolution of channel changes internally consistent within the  
385 model, timescales for morphological response will be much shorter than actual adjustment  
386 times in field settings because flood intermittency is not included (so the model as  
387 implemented is always at a constant flood discharge). In addition, the upstream sediment  
388 supply is imposed in the model, while in natural settings hillslope-floodplain-channel  
389 coupling could greatly affect  $q_{sfeed}$  over time if significant aggradation or downcutting took  
390 place.

391 Table 2 provides parameters used for morphodynamic modeling. Although the highly  
392 simplified model is not intended for quantitative field comparisons, variables  $D$  ( $D_{s50}=50$   
393 mm),  $f$  (0.1), and  $q_w$  ( $1 \text{ m}^2/\text{s}$ ) were chosen to be broadly consistent with a moderate ( $\approx 2\text{-}3$  year  
394 peak discharge recurrence interval) bedload-transporting flood in Reynolds Creek, Idaho  
395 (Olinde and Johnson, 2015). Reynolds creek is a snowmelt-dominated channel with reach  
396 slopes that vary widely from  $\sim 0.005$  to 0.07. In an instrumented reach with a slope of 0.02,  
397 Olinde (2015) used RFID-tagged tracers and channel-spanning RFID antennas to measure  
398  $\tau_{rs50}^* \approx 0.06$ . A constant  $\tau_c^* = 0.06$  is used for the Exner-only only models, while  $\tau_c^* = 0.06$  is



399 used as the initial condition for Exner+ $\tau_c^*$  models with evolving  $\tau_c^*$ . Upstream sediment feed  
400 rates ( $q_{sfeed}$ ) were not constrained by field data, and were chosen to provide reasonable  
401 modeled slopes. Exponents  $\kappa_{dep}$  and  $\kappa_{ent}$  used the experimental calibrations, while  $k$  were  
402 chosen so that changes in  $\tau_c^*$  occurred over the same range of timescales as topographic  
403 adjustments, to better illustrate the interplay of variables in morphodynamic evolution.

### 404 3.3.2 Morphodynamic model results

405 Fig. 6 compares how the Exner-only (constant  $\tau_c^*$ ) and Exner+ $\tau_c^*$  (evolving  $\tau_c^*$ )  
406 models respond to an increase in sediment supply. The initial condition is a channel at  
407 equilibrium ( $q_{sout}=q_{sin}=q_{sfeed}$ ). At  $t=0$ , sediment supply is increased by a factor 5. The  $\tau_c^*$ -  
408 evolution model aggrades to a new equilibrium slope that is lower than the constant  $\tau_c^*$  model.  
409 This occurs because  $\partial q_s / \partial x < 0$  causes evolving  $\tau_c^*$  to decrease over time, progressively  
410 increasing transport efficiency (i.e., higher transport rates at a lower slope) compared to  
411 constant  $\tau_c^*=0.06$  (Fig. 7). Feedback causes the reverse effect in response to a decrease in  
412  $q_{sfeed}$  (Fig. 7). For the Exner+ $\tau_c^*$  model,  $\tau_c^*$  progressively increases as slope decreases,  
413 leading the channel to re-equilibrate both sooner and at a higher slope.

414 An equilibrium timescale ( $t_{eq}$ ) is measured as the amount of time it takes from a  
415 supply perturbation ( $t=0$  in these models) to having the slope adjust to be within 0.0001 of its  
416 asymptotic equilibrium slope. In Fig. 7,  $t_{eq}$  are substantially longer for the Exner-only models  
417 (constant  $\tau_c^*$ ) than for the otherwise equivalent Exner+ $\tau_c^*$  models. Slope and  $\tau_c^*$  adjust at the  
418 same time in the Exner+ $\tau_c^*$  models, and influence transport in the same direction. For  
419 example, an increase in  $q_{sfeed}$  leads to aggradation, in turn increasing  $q_s^*$  by both increasing  
420 slope and decreasing  $\tau_c^*$  and (Eq. 1, 5). Both factors adjusting enable equilibrium to be  
421 reached sooner.

422 Fig. 8a confirms, over a  $q_{sfeed}$  range of two orders of magnitude, that equilibrium  
423 slopes changes less for the Exner+ $\tau_c^*$  model than for Exner-only. The ratio of these  
424 equilibrium slopes illustrates the magnitude of the change, where “ $S_{eq}$  ratio” is  $S_{eq}$  for  
425 Exner+ $\tau_c^*$  divided by Exner-only  $S_{eq}$  (Fig. 8b). An order-of-magnitude decrease in  $q_{sfeed}$





426 caused  $\text{Exner} + \tau_c^* S_{eq}$  to be between roughly 24% and 36% larger than Exner-only  $S_{eq}$ , while  
427 an order-of-magnitude increase in  $q_{sfeed}$  led to  $\text{Exner} + \tau_c^*$  roughly 20% smaller than the  
428 constant- $\tau_c^*$  model. Calculations are shown for several values of scaling factor  $k$ . A larger  $k$   
429 means that  $\tau_c^*$  increases or decreases more rapidly for a given amount of aggradation or  
430 degradation (Eq. 7), which in general enables a new equilibrium to be reached with a smaller  
431 change in slope.

432 Equilibrium timescales are quite sensitive to  $k$  as well as to sediment supply rate (Fig  
433 8c). Similar to the  $S_{eq}$  ratio, the “ $t_{eq}$  ratio” is  $\text{Exner} + \tau_c^* t_{eq}$  divided by Exner-only  $t_{eq}$  (Fig. 8d).  
434 There is an asymmetry in equilibrium times for aggradation vs. degradation; in general the  
435 difference between Exner-only and  $\text{Exner} + \tau_c^*$  is somewhat smaller during bed aggradation,  
436 and the difference decreases with increasing  $q_{sfeed}$ . Interestingly, the highest  $k$  ( $2.8E-5$ ) results  
437 in a threshold-like response where the  $t_{eq}$  ratio rapidly increases from roughly 0.01 to 0.8  
438 (Fig. 8d). This change occurred at the feed rate at which  $\tau_c^*$  “bottomed out”, i.e. reached its  
439 minimum possible value ( $\tau_c^* \approx \tau_{c\min}^* = 0.02$ ) before the equilibrium slope had been attained  
440 (Fig. 8e). At that point,  $\tau_c^*$  could no longer act as a buffer to reduce more gradual slope  
441 changes.

442 Finally, Fig. 9 shows that the spatial as well as temporal evolution of  $\tau_c^*$  can influence  
443 river profiles. The models are the same as in Fig. 6. At  $t=0$ , the feed rate *into the upstream-*  
444 *most node* (node 1, 0 km) increases by a factor of 5. Thus the upstream end feels the flux  
445 perturbation both sooner and more strongly than downstream nodes. Aggradation from the  
446 supply perturbation increases upstream slopes first. In the Exner-only model, downstream  
447 slopes gradually catch up (Fig. 9a): because  $\tau_c^*$  stays constant, every location along the  
448 channel eventually asymptotes to the single slope required to transport the new  $q_{sfeed}$  at the  
449 given discharge. However, for evolving  $\tau_c^*$ , enhanced upstream aggradation caused upstream  
450  $\tau_c^*$  to decrease both more rapidly and to lower values than downstream nodes. Spatial  
451 differences in  $\tau_c^*$  persisted at equilibrium, resulting in spatial variations in equilibrium slope  
452 ( $\text{Exner} + \tau_c^*$ ; Fig. 9b, 9c).

453



## 454 **4 Discussion**

455 In this section, the dependence of  $\tau_c^*$  on sediment supply is compared to previous  
456 work.  $\tau_c^*$  evolution is identified as a negative feedback on morphologic change that can  
457 impart a memory of previous channel “states” to the system. The physical significance of  $\tau_c^*$   
458 variability is interpreted in terms of steady-state slope variability along a natural channel.  
459 Finally,  $\tau_c^*$  is interpreted as a channel state variable, analogous to temperature in  
460 thermodynamics. By incorporating  $\tau_c^*$  evolution, landscape evolution models may be able to  
461 implicitly account for morphodynamic variables including roughness and form drag.

### 462 **4.1 Comparison to previous work**

463 Recking (2012) compared long-term bed load monitoring records from steep natural  
464 channels (>5% slope) to differences in sediment supply interpreted from aerial photographs of  
465 surrounding hillslopes. Channels with higher supply rates had higher transport rates for a  
466 given shear stress, suggesting a dependence of transport thresholds on supply. Recking  
467 (2012) proposed models for putting end-member bounds on reference stress for the cases of  
468 very high sediment supply ( $\tau_{mss}^*$ ) and very low sediment supply ( $\tau_m^*$ ) in steep mountain  
469 channels:

$$470 \quad \tau_{mss}^* = (5S + 0.06) \left( \frac{D_{84}}{D_{50}} \right)^{-1.5} \quad (19)$$

$$471 \quad \tau_m^* = (5S + 0.06) \left( \frac{D_{84}}{D_{50}} \right)^{4.4\sqrt{S}-1.5} \quad (20)$$

472 The ratio  $D_{84}/D_{50}$  is intended to represent the degree of armoring, which varies with sediment  
473 supply (e.g., Dietrich et al., 1989). It should be noted that these reference stresses parameters  
474 were calibrated to describe transport of the  $D_{84}$  grain size (rather than  $D_{50}$ ) using a different  
475 transport model (Recking, 2012). Nonetheless,  $\tau_{mss}^*$  and  $\tau_m^*$  were shown to be fairly  
476 comparable to typical nondimensional threshold such as  $\tau_c^*$  in Eq. (1). Fig. 10 compares Eq.  
477 (19) and (20) to the experimental constraints on  $\tau_{rs50}^*$ . For the most part,  $\tau_{mss}^*$  and  $\tau_m^*$  do  
478 bound  $\tau_{rs50}^*$ . The low supply bound  $\tau_m^*$  is roughly 3 times larger than the experimental



479 constraints. While  $\tau_{mss}^*$  is similar in magnitude to  $\tau_{rs50}^*$  and predicts the decrease during the  
480 feed period, the (linear) correlation between  $\tau_{mss}^*$  and  $\tau_{rs50}^*$  is weak ( $R^2=0.13$ ) although  
481 statistically significant ( $p=3E-5$ ). Nonetheless, given that threshold of motion uncertainties  
482 are typically large, Eq. (19) arguably provides a surprisingly good independent prediction of  
483 our experimental disequilibrium transport data, based on experimental slope,  $D_{84}$  and  $D_{50}$ .

484 Bunte et al. (2013) also interpreted that lower  $\tau_c^*$  corresponded to looser beds caused  
485 by higher rates of sediment supply from upstream, and noted that the stability of bed particles  
486 can be qualitatively assessed in the field while doing pebble counts. Yager et al. (2012b)  
487 demonstrated that in-channel sediment availability varied inversely with the degree of boulder  
488 protrusion. While the  $\tau_c^*$ -evolution model is not inconsistent with high sediment supply rates  
489 correlating with low  $\tau_c^*$ , Eq. (5) and (7) say something different:  $\tau_c^*$  does not directly increase  
490 or decrease with supply, but rather with the history of sediment supply relative to transport  
491 capacity over time. If  $q_{sin}$  equals  $q_{sout}$ ,  $\tau_c^*$  will remain constant regardless of whether  $q_{sin}$  is  
492 high or low.

#### 493 **4.2 Negative feedback and asymmetric approaches to equilibrium**

494 The evolution of  $\tau_c^*$  acts as a negative feedback because it reduces the  
495 morphodynamic response to perturbations. Reach slopes and  $\tau_c^*$  both change in the direction  
496 that brings transport back towards equilibrium, allowing smaller slope changes to accommodate  
497 supply changes (Fig. 6, 7, 8a,b, 9). However, as with other buffered systems, there is a limit to  
498 how large of a perturbation can be accommodated by  $\tau_c^*$  (as illustrated by  $k=2.8E-5$  in Fig.  
499 8c,d,e). The amount of possible  $\tau_c^*$  change depends on how close the  $\tau_c^*$  is to the threshold  
500 boundaries, i.e.  $\tau_{c\min}^*$  or  $\tau_{c\max}^*$  (Eq. 5, 7). When changes in  $\tau_c^*$  are no longer possible (or are  
501 asymptotically small) but transport and morphology are not equilibrated, then the time to  
502 equilibrium ( $t_{eq}$ ) increases because only channel morphology can adjust (Fig. 8c, d, e).

503 The experiments suggest that  $\tau_c^*$  changes faster in response to aggradation than  
504 degradation (Fig. 2, 5). This asymmetry is expressed in the best-fit exponents:  $\kappa_{dep}$  is smaller  
505 than  $\kappa_{ent}$  for all scenarios tested (Table 1). Note that because  $\partial\theta_s/\partial x$  is much smaller than 1  
506 (i.e. spatial changes in bed elevation are small compared to the horizontal distance the change



507 is measured over), the smaller exponent ( $\kappa_{dep}$ ) corresponds to a larger change in  $\tau_c^*$  for a  
508 given  $\partial\theta_s/\partial x$  (Eq. 7). Thus, aggradation is more efficient at decreasing  $\tau_c^*$  than degradation  
509 is at increasing  $\tau_c^*$ , for a given increment of sediment thickness ( $\theta_s$ ). But why then does Fig.  
510 8c indicate, for the Exner+ $\tau_c^*$  model, that equilibrium timescales are longer for aggradation  
511 ( $q_{sfeed} / \text{initial } q_{sfeed} > 1$ ) than for degradation? The explanation is that the equilibrium  
512 timescale does not *only* depend on the exponents, but also on how much total aggradation or  
513 degradation occurs to attain equilibrium, and how quickly sediment enters or exits the reach  
514 to enable that aggradation or degradation. More slope change occurred during aggradation  
515 than degradation for these particular Exner+ $\tau_c^*$  models (Fig. 8a), even though  $\tau_c^*$  also tended  
516 to change by more during aggradation than degradation (Fig. 8e).

517 In the experiments, average slopes changed very little in response to changes in  
518 sediment supply and transport disequilibrium, while grain size and bed surface roughness  
519 changed much more (Fig. 2). Because grain size changes were accounted for (by the W&CM)  
520 in determining experimental  $\tau_{rs50}^*$  (Fig. 3), roughness and other unquantified mechanisms  
521 (such as grain interlocking) are interpreted to have caused the  $\tau_{rs50}^*$  evolution. What does this  
522 suggest for  $k$ , which scales how much  $\tau_c^*$  changes for a given amount of aggradation or  
523 degradation? The best-fit  $k$  to the collective experiments was  $2.83E-3 \text{ s}^{-1}$ , which reflects the  
524 rapid adjustment of  $\tau_c^*$  compared to slope changes (Fig. 5, Table 1, Eq 7). In contrast, the  
525 morphodynamic modeling used  $k$  values adjusted to be 2 to 3 orders of magnitude smaller, so  
526 that the response to a perturbation in supply would involve non-negligible changes in slope  
527 (the only morphologic variable in the simple morphodynamic model) as well as in  $\tau_c^*$ . Higher  
528 values of  $k$  in the morphodynamic model cause  $\tau_c^*$  to adjust more rapidly and slope to adjust  
529 less (Fig. 8).

### 530 **4.3 Memory, morphologic variability, and Reynolds Creek**

531 An implication of  $\tau_c^*$  evolving with reach morphodynamics is that local channel form  
532 can retain “memories” of previous conditions, which can influence local responses to  
533 subsequent forcing. In Fig. 9b and 9c, an increase in supply led to the temporal and spatial  
534 evolution of  $\tau_c^*$ , which in turn caused spatial variations in equilibrium slope. Upstream



535 reaches acted as filters of the supply perturbation to downstream reaches (here, “reach” refers  
536 to a small fraction of the total modeled longitudinal distance). In contrast, slope variability  
537 developed during the transient adjustment period when  $\tau_c^*$  remained constant, but all reaches  
538 evolved to the same equilibrium slope required to transport the new supply (Fig. 9a).

539 Natural river channels inevitably exhibit morphologic variability at reach scales. For  
540 example, although the longitudinal profile of Reynolds Creek appears smoothly concave over  
541 a spatial scale of 10 km (Fig. 11a), there is substantial slope variability when calculated for  
542 100 m reaches (Fig. 11b). This 100 m averaging scale was chosen for the following analysis  
543 because it is sufficiently large to plausibly be used for landscape evolution modeling, while  
544 small enough to capture slope variability along a profile.

545 How much variability in reach slope could be explained by commonly observed  
546 variability in  $\tau_c^*$  (e.g., Fig. 1)? Eq. (18), which assumes equilibrium transport, can be  
547 rearranged to predict transport thresholds for a given slope, water discharge and  
548 nondimensional sediment flux:

$$549 \quad \tau_c^* = \frac{(q_w S)^{2/3}}{2D \left( \frac{\rho_s}{\rho} - 1 \right)} \left( \frac{f}{g} \right)^{1/3} - \left( \frac{q_s^*}{3.97} \right)^{2/3} \quad (21)$$

550 Fig. 11c compares a histogram of compiled  $\tau_c^*$  (Fig. 1) to a histogram of calculated  $\tau_c^*$  using  
551 Eq. (21), based on the reach slopes along Reynolds Creek (Fig. 11b). Fig. 11d compares  
552 slope predictions calculated using Eq. (18) and the  $\tau_c^*$  compilation (Fig. 1) to the 100 m reach  
553 slopes of Reynolds Creek. Although slope can be influenced by many factors, it appears  
554 plausible that natural slope variability in part reflects variability in thresholds of motion.  
555 While compilations are often assumed to reflect “typical” distributions, there is no reason that  
556 the  $\tau_c^*$  values from Fig. 1 should reflect the particular distribution of  $\tau_c^*$  for  $D_{50}$  in Reynolds  
557 Creek. Beyond a field measurement of 0.06 at a particular location,  $\tau_c^*$  distributions are  
558 spatially unconstrained. These calculations also assumed  $q_s^* = 2\text{E-}3$  because it provided a  
559 reasonable overlap of the distributions. Nonetheless, a hypothesis to motivate future work is  
560 that systematic and predictable differences in  $\tau_c^*$  would be found for Reynolds Creek reaches  
561 that overlap with the  $\tau_c^*$ -predicted slope distribution, because slope variations dominantly  
562 reflect adjustments to transport the sediment load from upstream. The steeper Reynolds Creek



563 slopes are interpreted to reflect additional slope influences, such as the abundance of  
564 boulders. Olinde (2015) did extensive point counts to measure surface GSDs over 11 reaches  
565 along Reynolds Creek, and found that coarse size percentiles (e.g.  $D_{84}$ ) varied greatly but  $D_{50}$   
566 had less variation. Larger boulders tend to be immobile, while the more mobile grains (e.g.  
567  $D_{50}$ ) are transported through all of the reaches (e.g., Yager et al., 2007).

568 Previous studies have explored the slope dependence of  $\tau_c^*$  by mechanistically  
569 explaining mean trends (Prancevic and Lamb, 2015; Lamb et al., 2008) (Fig 1). In addition,  
570 Fig. 3, 9 and 11 suggest that  $\tau_c^*$  variability can be both meaningful and physically inherent to  
571 bedload transport, because slope and  $\tau_c^*$  inevitably evolve together in response to discharge  
572 and sediment supply. In nature, spatially and temporally-averaged morphodynamic  
573 equilibrium will reflect “channel-forming” discharges and a representative sediment supply  
574 from upstream, but floods, local supply perturbations and history add to local variability in  $\tau_c^*$   
575 and morphology.

#### 576 **4.4 State function framework for modeling morphodynamics**

577 I argue that  $\tau_c^*$  is an important state variable for gravel-bed channels, and outline a  
578 possible state function approach for modeling the morphodynamic evolution of channels. The  
579 term “bed state” has long been informally used to describe collective aspects of local channel  
580 morphology, such as surface GSD and armoring and clustering, that change with relative ease  
581 and influence transport rates (e.g., Church, 2006; Gomez and Church, 1989). Although  
582 explicitly describing  $\tau_c^*$  evolution and related river feedbacks in terms of state and path  
583 functions may be novel (to my knowledge), this approach is in many ways conceptually  
584 equivalent to the description by Phillips (2007) of landscape evolution and form in terms of  
585 system states and the importance of history, and similar to other works that describe and  
586 generalize complex channel morphology process and response feedbacks (e.g., Fonstad, 2003;  
587 Phillips, 2011, 2009; Chin and Phillips, 2007; Phillips, 1991; Stark and Stark, 2001; Yanites  
588 and Tucker, 2010).

589 State variables or state functions are integral to many disciplines, including control  
590 systems engineering and thermodynamics. For example, temperature, pressure, enthalpy and  
591 entropy are some of the many thermodynamic state variables. By definition state variables are  
592 path-independent (Oxtoby et al., 2015). For example, temperature ( $T$ ) describes the amount



593 of thermal energy per unit of a material. A change in temperature of the material depends on  
594 its initial and final states alone (i.e.,  $\Delta T = T_2 - T_1$ ), but does not depend on the path, i.e. the  
595 history of temperatures in between times  $t_2$  and  $t_1$ . In contrast, heat--the flow (transfer) of  
596 thermal energy--is a path function (or process function), not a state variable. Heat flow  
597 between bodies is both controlled by and changes the temperature (the state) of those bodies,  
598 but the amount of total heat transferred does depend on the path. Three other points about  
599 state functions are relevant. First, state functions are often interrelated rather than independent  
600 of one another. For example, Gibbs free energy is state function calculated from temperature,  
601 enthalpy and entropy (Hemond and Fechner, 2014). Second, although traditional state  
602 functions are technically only defined for systems at equilibrium, in practice they are valid  
603 and useful approximations of gradually evolving systems (e.g., Kleidon, 2010). Third, the  
604 evolution of systems involving multiple state variables are usually described with coupled  
605 differential equations.

606 Channel morphodynamics could be described by a similar framework of state and path  
607 functions. Analogous to heat, the cumulative discharges of both water and sediment are path  
608 functions that drive bed state evolution. Channel morphology can be described by numerous  
609 bed state variables, including but not limited to surface GSD, slope, width, depth, bed  
610 roughness, surface grain clustering, interlocking, overlap and imbrication, and finally  $\tau_c^*$ .  
611 Analogous to temperature,  $\tau_c^*$  is a state variable because, for example, a change in  $\tau_c^*$  of 0.01  
612 (from say 0.04 at  $t_1$  to 0.05 at  $t_2$ ) does not depend on the progression of values in between.  
613 However, the amount of sediment transported between times  $t_1$  and  $t_2$  does depend on the  
614 history of  $\tau_c^*$ , and also influences the history of  $\tau_c^*$  (Eq. 5, 7).

615 It is worth noting that entropy is the state variable perhaps used most often to  
616 characterize channel systems (e.g., Chin and Phillips, 2007; Leopold and Langbein, 1962;  
617 Rodriguez-Iturbe and Rinaldo, 1997). Entropy is often used to provide a closure for  
618 underconstrained sets of equations, by assuming that geomorphic and other systems  
619 inherently maximize their entropy at equilibrium (Kleidon, 2010; Chiu, 1987). A limitation of  
620 some maximum-entropy landscape models is that physically-based surface processes are not  
621 always explicitly modeled, making results difficult to validate and less useful for exploring  
622 landscape responses to perturbations or behavior away from steady state, even if they can



623 create reasonable-looking equilibrium morphologies (Paik and Kumar, 2010). In contrast,  
624 state function  $\tau_c^*$  has a clear process-based meaning.

625 How could a state function framework improve morphodynamic modeling and  
626 incorporate subgrid-scale channel feedbacks into broader landscape evolution models?  
627 Simplicity is important: the most insight is often gained by using the simplest possible model  
628 that can still capture essential feedbacks over spatial and temporal scales of interest. As  
629 described in the introduction, previous work has shown that  $\tau_c^*$  is influenced by many  
630 characteristics of the surrounding bed surface. In other words,  $\tau_c^*$  is closely related to other  
631 bed state variables. It would probably be impractical to develop or apply a “complete”  $\tau_c^*$   
632 model that explicitly incorporated separate state variables for multiple known controls on  $\tau_c^*$ .  
633 Instead, the  $\tau_c^*$  evolution equation (Eq. 5, 7) attempts to strike a balance between predicting  
634 process in a physically justifiable (but empirically calibrated) way, while remaining broadly  
635 applicable. It could similarly be impractical to apply a “complete” reach-scale  
636 morphodynamic model that explicitly parameterized myriad feedbacks using every  
637 corresponding bed state variable. Instead, because many bed state variables are not actually  
638 independent of  $\tau_c^*$ , aspects of morphology can be subsumed into evolving  $\tau_c^*$  for modeling  
639 purposes. This is similar to the channelization approach of Stark and Stark (2001). For  
640 example, in the experiments the bed responded to transport disequilibrium primarily by  
641 changing roughness but not slope. However, roughness was not an explicit, separate variable  
642 in the best-fit  $\tau_{rs50}^*$  calculations. Instead, some effects of evolving roughness and other bed  
643 state controls (imbrication, clustering) on transport rates became implicitly accounted for in  
644 the experimentally calibrated  $\tau_{rs50}^*$  ( $\approx \tau_c^*$ ). The best-fit model parameters ( $k$ ,  $\kappa_{dep}$ ,  $\kappa_{ent}$ ; Eq. 7;  
645 Table 1) would presumably change if a separate differential equation was developed to  
646 explicitly describe another state variable that is currently implicit in  $\tau_c^*$ , such as bed  
647 roughness. It is also important to note that bed roughness could have other effects, such as  
648 modifying hydraulic roughness, that are not implicitly captured by  $\tau_{rs50}^*$  (Schneider et al.,  
649 2015b).





#### 650 **4.5 Form drag vs. parsimony**

651 Calculations of best-fit  $\tau_{rs50}^*$  and transport rates used total shear stress (Eq. 9), rather  
652 than partitioning stress into form drag and a lower effective stress used for calculating  
653 transport rates (skin friction). Although not a state variable, form drag is physically justifiable  
654 because larger clasts that protrude higher into the flow (e.g. stable boulders) tend to account  
655 for a disproportionate amount of the total stress through drag, turbulence generation and  
656 pressure gradients. Form drag corrections have been incorporated into many transport models  
657 to enable reasonable transport rates to be calculated using  $\tau_c^*$  values typical of systems  
658 without form drag (e.g., Rickenmann and Recking, 2011; David et al., 2011; Yager et al.,  
659 2012a). Conversely, another common approach is simply to use higher  $\tau_c^*$  (e.g., Bunte et al.,  
660 2013; Lenzi et al., 2006), consistent with acknowledging that  $\tau_c^*$  can be a physically  
661 meaningful fitting parameter to predict transport. Using field data on steep channel gravel  
662 transport, Schneider et al. (2015a) recently compared transport predictions based on (a) form  
663 drag corrections and (b) higher reference stresses. For the most part they found that either  
664 approach could provide similar accuracy. They also noted that “uncertainties in predicted  
665 transport rates remain huge (up to roughly 3 orders of magnitude)” (Schneider et al., 2015a),  
666 and suggested that factors including supply effects may account for remaining discrepancies.  
667 Although beyond the scope of the present analysis, form drag effects could be separated from  
668 best-fit  $\tau_{rs50}^*$  by using a calculated skin friction stress rather than total stress. However, doing  
669 so would add extra uncertainty to the shear stresses, while still not accounting for effects of  
670 sediment supply. Implicitly subsuming form drag into  $\tau_c^*$  arguably provides a simpler and  
671 more parsimonious approach for modeling transport and morphodynamics.

#### 672 **5 Conclusions**

673 Flume data and a corresponding model demonstrate that nondimensional critical shear  
674 stress ( $\tau_c^*$ ) evolves through time as a function of sediment transport disequilibrium. Net  
675 erosion tends to increase local  $\tau_c^*$  (reducing transport rates), while net deposition tends to  
676 decrease  $\tau_c^*$  (increasing transport rates). This  $\tau_c^*$  dependence on sediment supply relative to  
677 transport capacity can plausibly explain much of the ~order-of-magnitude variability almost  
678 inevitably observed in transport threshold data (Fig. 1). This view contrasts with a more



679 conventional approach in which thresholds of motion are assumed to remain constant in time,  
680 and  $\tau_c^*$  variability is treated as noise rather than interpretable signal.

681 Flume experiments measured sediment transport and channel response away from  
682 equilibrium, as beds stabilized and responded to pulses of gravel. Because studies of  
683 transport disequilibrium are relatively uncommon, much of our ability to predict transport  
684 rates implicitly assumes steady state conditions. The experiments explore a parameter space  
685 typical of step-pool morphologies, with steep slopes (8-12%), flow depths comparable to the  
686 coarsest grain size fractions, and mixed grain size transport (Johnson et al., 2015). While  $\tau_c^*$   
687 has physical meaning, it is also a fitting parameter that depends on the choice of sediment  
688 transport model. The experimental data are fit by the Wilcock and Crowe (2003) model for  
689 mixed grain size transport, to back-calculate both hiding function exponents and transport  
690 thresholds (which in this model are nondimensional reference stresses,  $\tau_{rs50}^*$ ). The Wilcock  
691 and Crowe (2003) hiding function provides a good prediction of our data (including different  
692 exponents for relatively small and large grains), supporting its applicability to steep channels.

693 A new differential equation is proposed for predicting the temporal evolution of  $\tau_c^*$  as a  
694 function of  $\partial q_s / \partial x$  (Eq. 5-7). After empirically fitting three model parameters, this  $\tau_c^*$ -  
695 evolution equation can explain nearly 70% of the variability in experimental  $\tau_c^*$ . We  
696 incorporate the  $\tau_c^*$ -evolution equation into a simple model for channel slope evolution.  
697 Changes in  $\tau_c^*$  are negative feedbacks on morphodynamic response, because not only slope  
698 but also  $\tau_c^*$  evolve when perturbed. Evolving  $\tau_c^*$  also reduces response timescales, allowing  
699 channels to more rapidly adjust to new conditions. In addition, spatial and temporal  
700 variations in sediment supply can lead to reaches becoming equilibrated over a range of  
701 slopes, because  $\tau_c^*$  also influences the equilibrium slope.

702 Finally,  $\tau_c^*$  is interpreted to be a state variable for fluvial channels. State functions and  
703 path functions are fundamental to many disciplines such as thermodynamics, because they  
704 allow the evolution of systems to be calculated. The same should be true for channels;  
705 sediment transport is to heat as  $\tau_c^*$  is to temperature. More broadly, conceptualizing  
706 morphodynamic and landscape evolution models in terms feedbacks among evolving state



707 variables and path functions may improve our ability to predict landscape responses to land  
708 use, climate change and tectonic forcing.

709

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715

### 716 **Appendix 1 List of variables**

717  $A_r$  Dimensionless parameter for incorporating grain size or roughness ratios in  
718 Eq. (7) [1]

719  $b$  Dimensionless hiding function exponent; either described by Eq. 13 or fit as a  
720 single value [1]

721  $B$  Dimensionless “feedback factor”; Eq. 6 [1]

722  $c_1, c_2, c_3$  Dimensionless empirical constants in Eq. (14) [1]

723  $D$  Grain diameter, for model cases with a single size only [L]

724  $D_{50}$  Median grain diameter [L]

725  $D_{s50}$  Median grain diameter of bed surface [L]

726  $D_i$  Grain diameter of size class  $i$  [L]

727  $f$  Darcy-Weisbach hydraulic friction coefficient; Eq. (15) [1]

728  $Fr$  Froude number [1]

729  $F_i$  Areal fraction of grain size class  $i$  on the bed surface; Eq. 10 [1]

730  $F_s$  Areal fraction of sand on the bed surface; Eq. 14 [1]

731  $g$  Gravitational acceleration [ $LT^{-2}$ ]

732  $h$  Water depth [L]



733	$\kappa_{dep}$	Exponent for net deposition in $\tau_c^*$ -evolution models; Eq. (5), (7). [1]
734	$\kappa_{ent}$	Exponent for net erosion in $\tau_c^*$ -evolution models; Eq. (5), (7). [1]
735	$k$	Scaling factor for $\tau_c^*$ evolution. Dimensions are [1/T] for Eq. (7)
736	$\lambda_p$	Bed porosity [1]
737	$q_{bi}$	Volume sediment flux per unit width of size class $i$ in Wilcock and Crowe
738		(2003); Eq. 10 [ $L^2/T$ ]
739	$q_s$	Volume sediment flux per unit width [ $L^2/T$ ]
740	$q_s^*$	Nondimensional volume sediment flux; Eq. (1) [1]
741	$q_{sin}$	Sediment flux entering a channel bed area (reach) of interest [ $L^2/T$ ]
742	$q_{sout}$	Sediment flux exiting a channel bed area (reach) of interest [ $L^2/T$ ]
743	$q_{sfeed}$	Sediment flux entering upstream end of overall model domain [ $L^2/T$ ]
744	$q_w$	Volume water discharge per unit width [ $L^2/T$ ]
745	$\rho$	Water density [ $M/L^3$ ]
746	$\rho_s$	Sediment density [ $M/L^3$ ]
747	$S$	Water surface slope [1]
748	$S_{eq}$	Water surface slope when reach is at equilibrium [1]
749	$\sigma$	Bed roughness, measured here as the standard deviation of detrended bed
750		elevations [L]
751	$\theta_s$	Thickness of sediment deposited or eroded in a time interval; Eq. (7) [L]
752	$t$	Time [T]
753	$t_{eq}$	Equilibrium timescale for morphological adjustment [T]
754	$\tau$	Shear stress [ $MT^{-2}L^{-1}$ ]
755	$\tau^*$	Shields stress (nondimensional shear stress) [1]



756	$\tau_c^*$	Critical Shields stress (nondimensional critical shear stress) [1]
757	$\tau_{c\max}^*$	Imposed maximum bound for $\tau_c^*$ in Eq. (5), (7) [1]
758	$\tau_{c\min}^*$	Imposed minimum bound for $\tau_c^*$ in Eq. (5), (7) [1]
759	$\tau_{mss}^*$	High sediment supply nondimensional reference stress end-member bound in
760		Recking (2012) transport model; Eq. (19) [1]
761	$\tau_m^*$	Low sediment supply nondimensional reference stress end-member bound in
762		(Recking, 2012) transport model; Eq. (20) [1]
763	$\tau_{ri}^*$	Reference Shields stress for size class $i$ , from Wilcock and Crowe (2003) [1]
764	$\tau_{rs50}^*$	Nondimensional reference Shields stress for surface grains of size $D_{s50}$ [1]
765	$U$	Depth-averaged water velocity [L]
766	$u_\tau$	Shear velocity; Eq. (10) [L/T]
767	$x$	Position measured horizontally (distance along channel) [L]
768	$z$	Position measured vertically (bed elevation)[L]
769	$W_i^*$	Nondimensional bedload transport rate for grain size class $i$ , in Wilcock and
770		Crowe (2003) [1]
771	W&CM	Abbreviation for Wilcock and Crowe (2003) transport model.

772

### 773 Captions

774 Figure 1. Threshold of motion data from both field and experimental studies. A power law  
775 regression to these data gives  $R^2=0.34$ , indicating that a majority of the variability is not  
776 explained by slope alone. Dotted lines indicate common range of  $\tau_c^*=0.03$  to 0.06 often  
777 assumed for modeling transport, although measured data fall well out of this range. Data  
778 have been additionally filtered to only include  $D_{50} > 2$  mm (i.e. gravel) and slopes between  
779 0.002 and 0.2. Data were compiled and provided by Prancevic and Lamb (2015), based in  
780 part on Buffington and Montgomery (1997), with additional data from Olinde (2015) and  
781 Lenzi et al. (2006).



782

783 Figure 2. Flume experiment data (Johnson et al., 2015). a. Sediment transport rate in ( $Q_{sfeed}$ )  
784 and out of the flume. The upstream sediment supply rate was zero other than during the  $Q_{sfeed}$   
785 period. Experiment 1 was run for a longer duration than the others but shows similar trends.  
786 Note that the outlet  $Q_s$  adjusts much faster to match the increase in supply than it does to  
787 decrease during periods of no input. b. Median bed surface grain diameters decreased during  
788 the feed of finer gravel, and then increase beyond their previous stable bed. c. Flume-  
789 averaged bed slopes changed relatively little even as transport rates and  $D_{50}$  changed greatly  
790 in response to initial bed stabilizing and supply perturbations.

791

792 Figure 3.  $\tau_{rs50}^*$  fits to the experimental data with the W&CM. , “W&C fit“ uses Eq. (13) to  
793 calculate hiding function exponent b, while “Power-law fit“ calculates a best-fit b along with  
794  $\tau_{rs50}^*$ . Error bars give 95% confidence intervals on  $\tau_{rs50}^*$  based on the regressions; although  
795 uncertainty can be broad the trends are clear and consistent. Shaded area indicates times of  
796 fine gravel addition (sediment feed) in each experiment.

797

798 Figure 4. Data points are based on power-law fits to exponent b. The W&CM hiding function  
799 (Eq. 13) does a good job matching the data, although it was not fit to these points. The first 6  
800 measurements of each experiment (roughly the first 10 minutes) were excluded because of  
801 large scatter associated with the greatest bed instability. The plot axes reflect the left and  
802 right hand sides of Eq. (12), although the plot uses dimensional stresses to be consistent with  
803 plots shown in Wilcock and Crowe (2003).

804

805 Figure 5. Best-fit models (Eq. 7 and 14) compared to experimental constraints. The periods  
806 of upstream sediment supply ( $Q_{sfeed}$ ) are indicated by the grey boxes for each experiment.

807

808 Figure 6. Profile evolution, comparing the morphodynamic responses of models with and  
809 without threshold evolution. The initial condition is an equilibrium channel with  $\tau_c^*=0.06$ ,  
810 upstream sediment supply  $q_s=1e-3$  m<sup>2</sup>/s, and an initial equilibrium slope of 0.0147. Sediment



811 supply is increased 5x at  $t=0$ . Lines are each 5 model days apart, and indicate the evolution to  
812 a new transport equilibrium.

813

814 Figure 7. Slope and critical shear stress evolution, for sediment supply increases (which  
815 correspond to Fig. 6 models) and decreases by factors of 5. As in figure 6,  $t=0$  corresponds to  
816 an equilibrium condition where the initial slope and initial threshold are consistent with the  
817 initial upstream sediment supply. Slope and  $\tau_c^*$  were averaged over nodes 3-10, leaving out  
818 the first and last two nodes because of minor model boundary effects.

819

820 Figure 8. Morphodynamic model sensitivity to sediment supply perturbations and  $k$ . All  
821 models started at the same equilibrium condition as shown in Fig. 6 and 7. a. Slope  
822 adjustment, normalized by the initial equilibrium slope. The correspondence of Eq. 17 and the  
823 morphodynamic model calculations demonstrate that the models did asymptotically attain  
824 equilibrium slopes. b.  $S_{eq}$  ratio is the ratio of equilibrium slopes of the Exner+ $\tau_c^*$  model  
825 divided by  $S_{eq}$  for the Exner-only model, to show the relative affect that that  $\tau_c^*$  evolution has  
826 on equilibrium slopes. c. Equilibrium timescales for model adjustment. d.  $t_{eq}$  ratio is the ratio  
827 of  $t_{eq}$  for the Exner+ $\tau_c^*$  model divided by  $t_{eq}$  for the Exner-only model. Values are lower than  
828 1, indicating that the  $\tau_c^*$  evolution has a large influence on equilibrium timescales. e.  
829 Evolution of  $\tau_c^*$ .

830

831 Figure 9. Spatial and temporal evolution of morphodynamic slopes, for the same models  
832 shown in Fig. 6. Slope is initially at equilibrium and responds to the 5x increase in upstream  
833 sediment supply at  $t=0$ . a. The Exner-only model initially has spatial slope variability, but  
834 evolves to a uniform new equilibrium slope. b, c. In the model with evolving  $\tau_c^*$ , slope and  
835  $\tau_c^*$  variability persist even at equilibrium.

836



837 Figure 10. Comparison of experimental and best-fit model constraints on  $\tau_{rs50}^*$ , compared to  
838 proposed constraints for D84 reference stress bounds for low and high sediment supply from  
839 Recking (2012).

840

841 Figure 11. Comparison of Reynolds creek and  $\tau_c^*$  data compilation predictions using Eq. (18)  
842 and (21). a. Longitudinal profile based on airborne Lidar (Olinde, 2015). Upstream end  
843 (distance=0) is an arbitrary location along the channel (a bridge). Data gap at 730 m is a  
844 gauging station weir; the slope steepens downstream of the weir where the valley becomes  
845 constricted by bedrock, although the bed remains almost entirely alluvial. b. Slopes  
846 calculated (averaged) over 100 m reaches, illustrating reach-scale slope variability. c.  
847 Histogram of  $\tau_c^*$  compilation (data shown in Fig. 1), compared to  $\tau_c^*$  calculated using Eq.  
848 (21), based on Reynolds Creek 100 m slopes (panel b), and assuming  $q_w=1$  m<sup>2</sup>/s,  $q_s^*=2e-3$ ,  
849 and  $f=0.1$ . d. Similar calculation as panel c, but using Eq. 18 to solve for slope as a function  
850 of  $\tau_c^*$  from the Fig. 1 data compilation, and compared to panel b slopes.

851

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Table 1. Best-fit threshold evolution models

Model type		Best-fit coefficients			
		$c_1$	$c_2$	$c_3$	$R^2$
"Sand fraction" $F_i < 2.8$ mm, collective best fit Eq. 14		0.097 (0.057, 0.14) <sup>a</sup>	0.103 (0.009, 0.22) <sup>a</sup>	(- 5.47 (-16.8, 5.8)) <sup>a</sup>	0.13
Units		$k$	$K_{dep}$	$K_{ent}$	$R^2$
		1/s	1	1	
$\partial\theta_s/\partial x$ model, collective fit	Eq. 7, $A_r=1$	2.83E-03	0.2	0.4	0.69
$\partial\theta_s/\partial x$ model, Expt 1 fit	Eq. 7, $A_r=1$	4.12E-02	0.55	0.61	0.52
$\partial\theta_s/\partial x$ model, Expt 2 fit	Eq. 7, $A_r=1$	4.80E-02	0.55	0.73	0.73
$\partial\theta_s/\partial x$ model, Expt 3 fit	Eq. 7, $A_r=1$	2.83E-03	0.22	0.41	0.77
$\partial\theta_s/\partial x$ model, Expt 4 fit	Eq. 7, $A_r=1$	2.75E-02	0.43	0.62	0.75
$\partial q_s/\partial x$ model, collective fit	Eq. 5	9.83E-03 <sup>b</sup>	0.25	0.4	0.69
$\partial\theta_s/\partial x$ model, collective fit	Eq. 7, $A_r=D_{s50}/\sigma$	4.17E-03	0.24	0.43	0.69
$\partial\theta_s/\partial x$ model, collectivet fit	Eq. 7, $A_r=D_{s50}/D_{s84}$	4.83E-03	0.23	0.41	0.69
$\partial\theta_s/\partial x$ model, collective fit	Eq. 7, $A_r=2D_{s50}/(D_{s84}-D_{s16})$	3.83E-03	0.23	0.42	0.69

<sup>a</sup>Confidence intervals are +-95%, based on nonlinear multiple regression in Matlab

<sup>b</sup>Units on k for Eq. 5 vary with kent and kdep



Table 2. Morphodynamic Model Parameters  
 Constant independent variables:

		Units
$q_w$	1.00	m <sup>2</sup> /s
$D_{50}$	50	mm
$f$	0.1	
$h$	0.50	m
$U$	1.99	m/s
$\lambda_p$	0.25	
$\kappa_{dep}$	0.2	
$\kappa_{ent}$	0.4	
$\tau_{cmin}^*$	0.02	
$\tau_{cmax}^*$	0.35	
$\rho$	1000	kg/m <sup>3</sup>
$\rho_s$	2600	kg/m <sup>3</sup>
Total duration	0.5	years
# nodes	12	
node spacing	100	m
$U_{bedload}/U$	0.5	

Initial condition:		Units
$q_{sin, init}$	1.00E-03	m <sup>2</sup> /s
$S, init$	0.0147	
$\tau_{cr}^*, init$	0.06	
Variables changed:		
$q_{sin}$	1E-5 to 1E-1	m <sup>2</sup> /s
$k$	2.8E-6, 5.7E-6, 2.8E-5	1/s

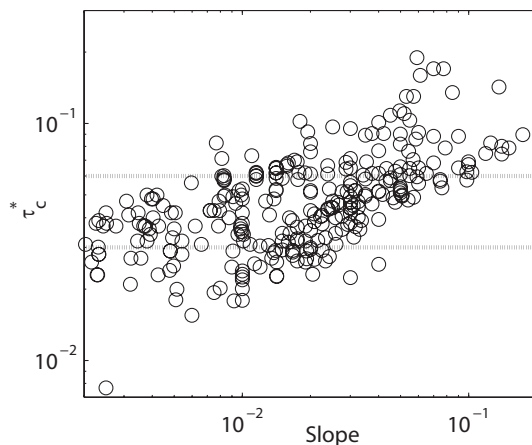


Figure 1. Threshold of motion data from both field and experimental studies. A power law regression to these data gives  $R^2=0.34$ , indicating that a majority of the variability is not explained by slope alone. Dotted lines indicate common range of 0.03 to 0.06 often assumed for modeling transport, although measured data fall well out of this range. Data have been additionally filtered to only include  $D_{50} > 2$  mm (i.e. gravel) and slopes between 0.002 and 0.2. Data were compiled and provided by Prancevic and Lamb (2015), based in part on Buffington and Montgomery (1997), with additional data from Olinde (2015) and Lenzi et al. (2006).

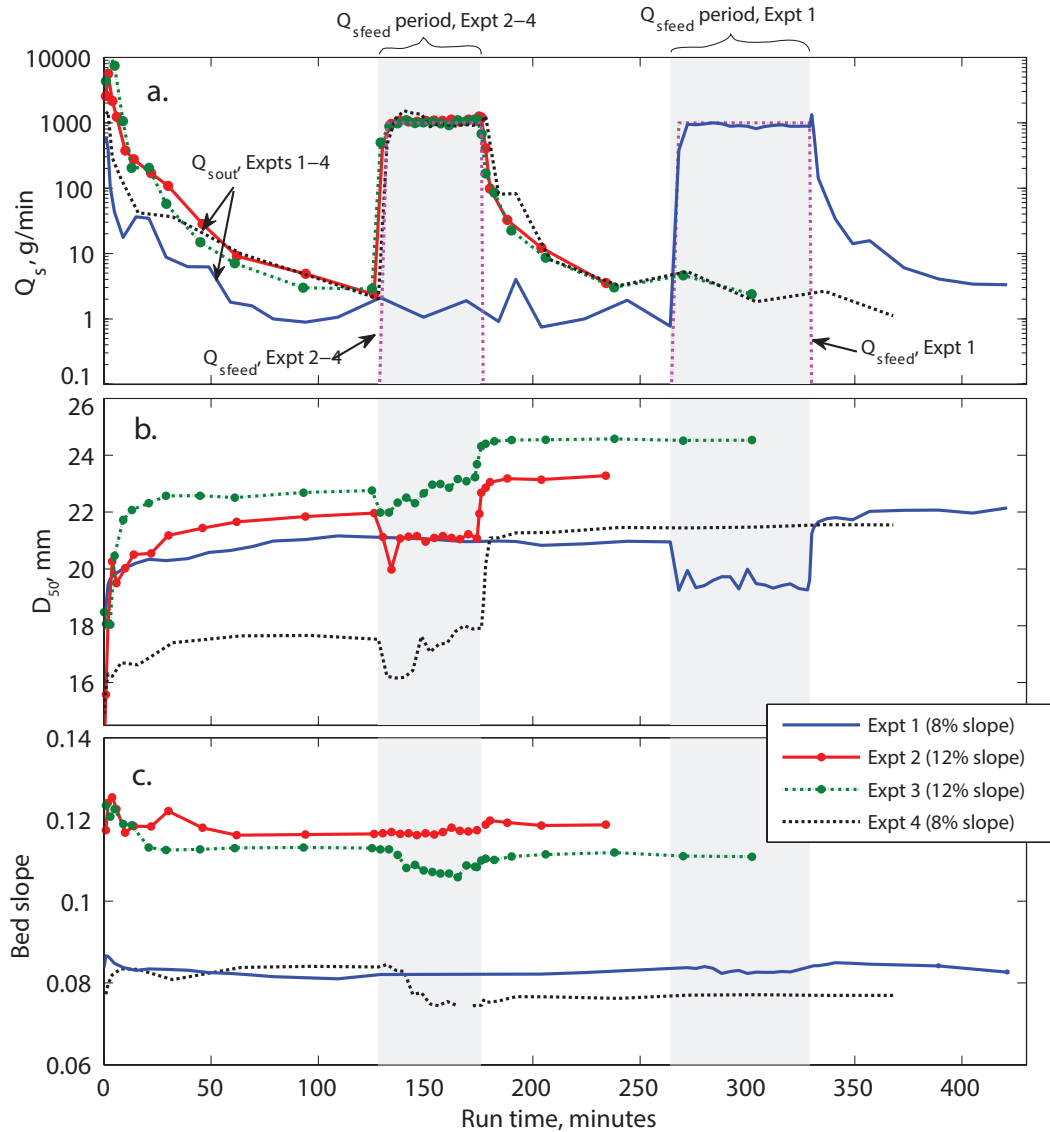


Figure 2. Flume experiment data (Johnson et al., 2015). a. Sediment transport rate in ( $Q_{s,feed}$ ) and out of the flume. The upstream sediment supply rate was zero other than during the  $Q_{s,feed}$  period. Experiment 1 was run for a longer duration than the others but shows similar trends. Note that the outlet  $Q_s$  adjusts much faster to match the increase in supply than it does to decrease during periods of no input. b. Median bed surface grain diameters decreased during the feed of finer gravel, and then increase beyond their previous stable bed. c. Flume-averaged bed slopes changed relatively little even as transport rates and  $D_{50}$  changed greatly in response to initial bed stabilizing and supply perturbations.



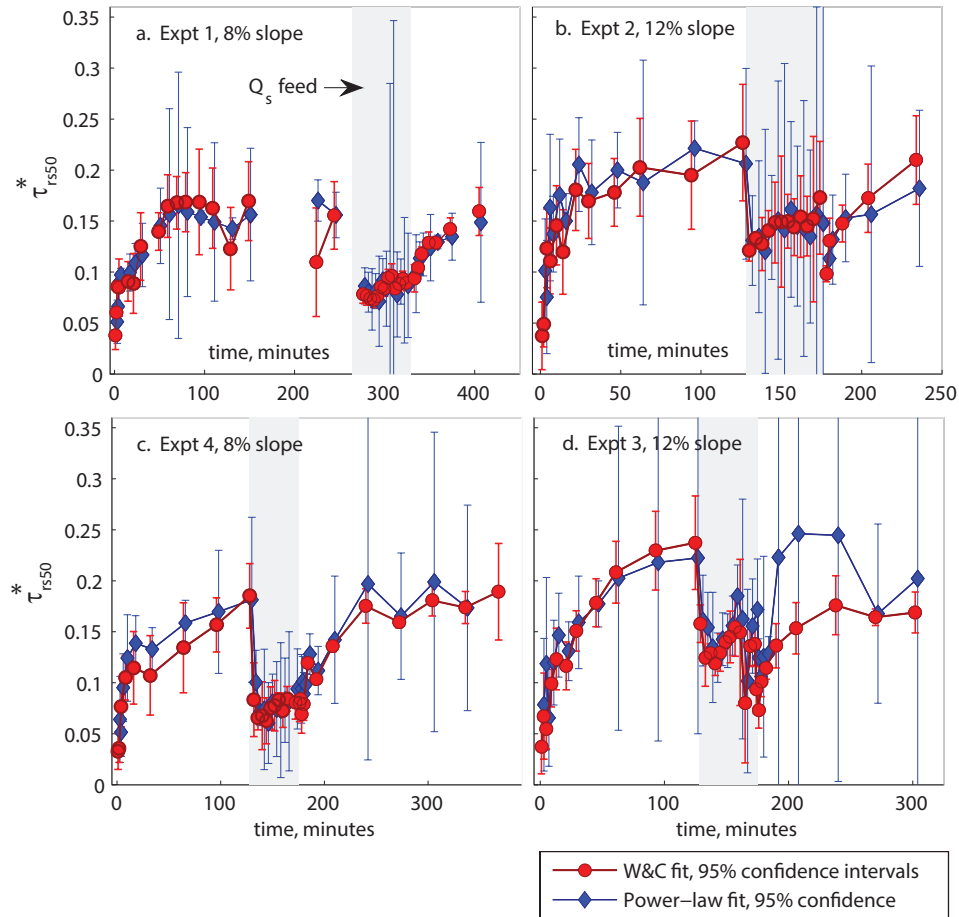


Figure 3. Fits to the experimental data with the W&CM. , “W&C fit“ uses Eq. (13) to calculate hiding function exponent  $b$ , while “Power-law fit“ calculates a best-fit  $b$  along with the threshold parameter. Error bars give 95% confidence intervals based on the regressions; although uncertainty can be broad the trends are clear and consistent. Shaded area indicates times of fine gravel addition (sediment feed) in each experiment.

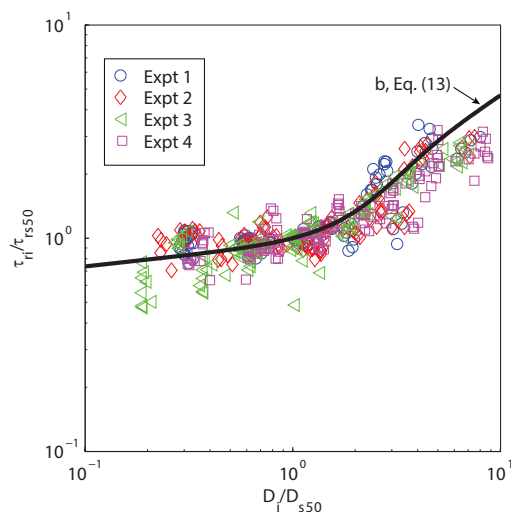


Figure 4. Data points are based on the “Power-law” fits for exponent  $b$ . The W&CM hiding function (Eq. 13) does a good job matchign the data, although it was not fit to these points. The first 6 measurements of each experiment (roughly the first 10 minutes) were excluded because of large scatter associated with the greatest bed instability. The plot axes reflect the left and right hand sides of Eq. (12), although the plot uses dimensional stresses to be consistent with plots shown in Wilcock and Crowe (2003).

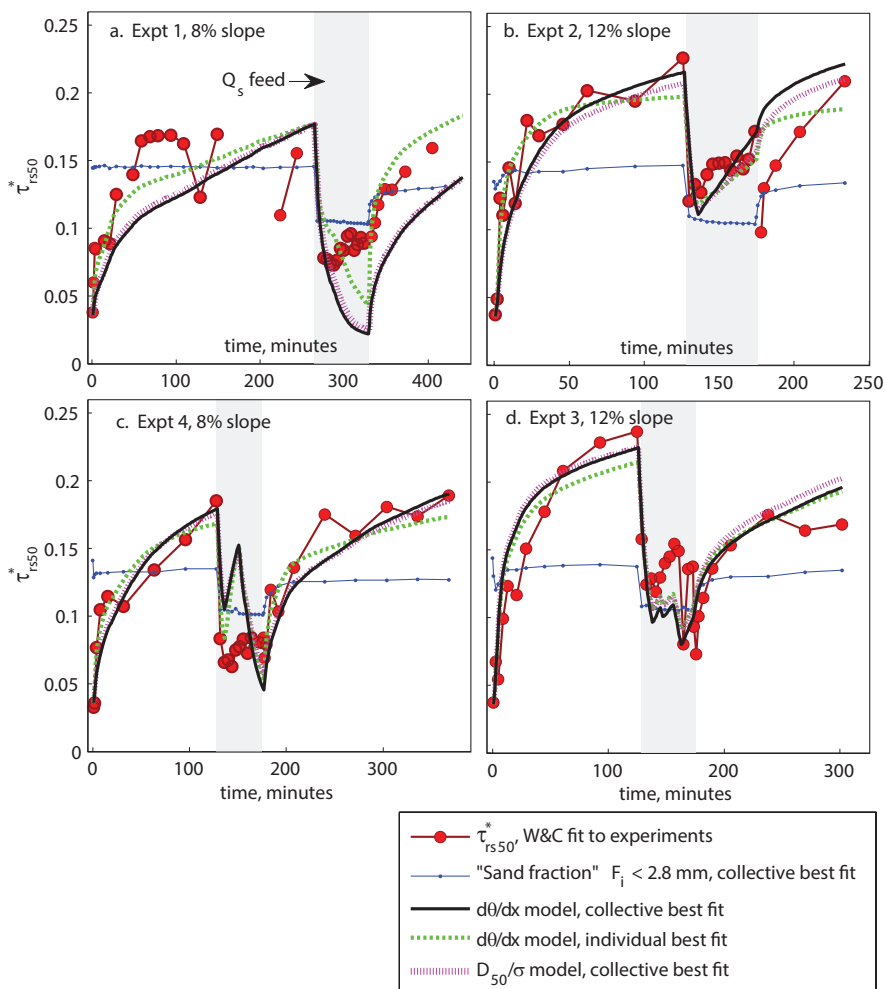


Figure 5. Best-fit models (Eq. 7 and 14) compared to experimental constraints. The periods of upstream sediment supply ( $Q_{sfeed}$ ) are indicated by the grey boxes for each experiment.

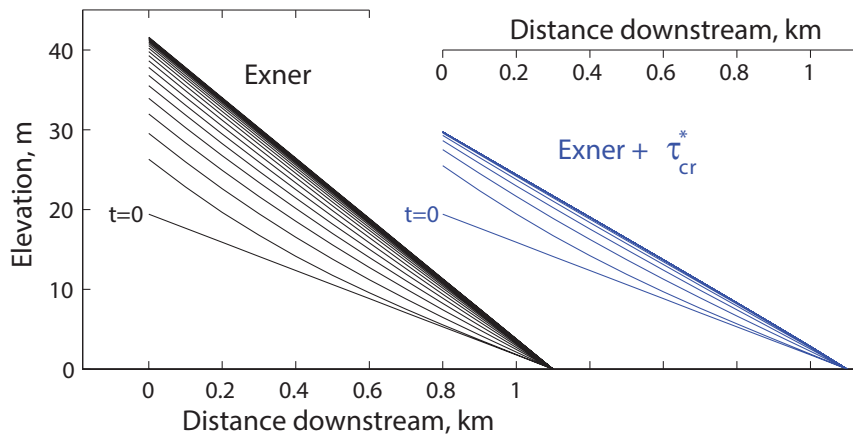


Figure 6. Profile evolution, comparing the morphodynamic responses of models with and without threshold evolution. The initial condition is an equilibrium channel with  $\tau_c^*=0.06$ , upstream sediment supply  $q_s=1e-3$  m<sup>2</sup>/s, and an initial equilibrium slope of 0.0147. Sediment supply is increased 5x at  $t=0$ . Lines are each 5 model days apart, and indicate the evolution to a new transport equilibrium.

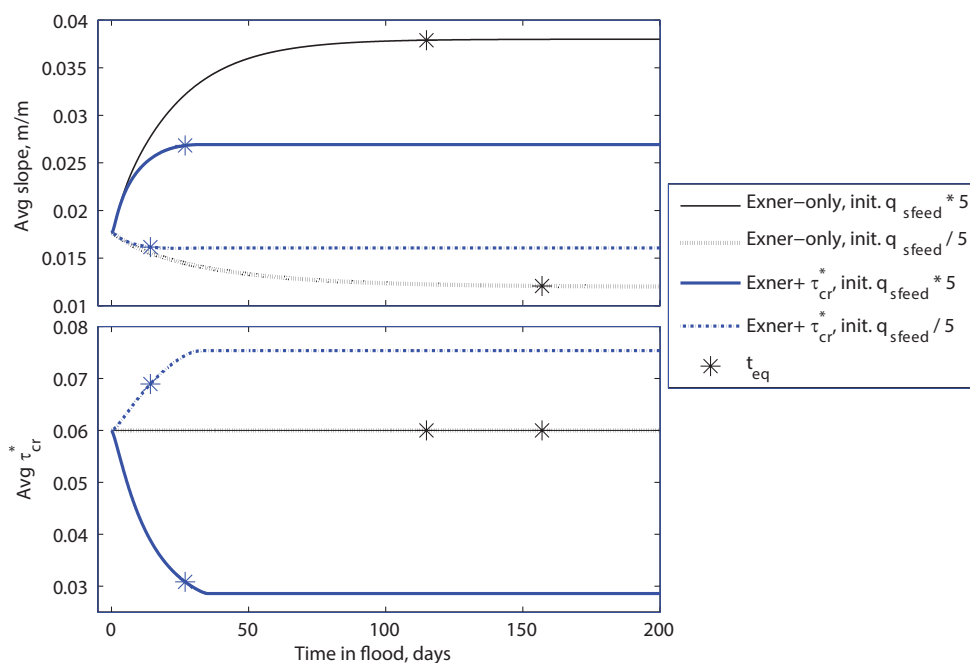


Figure 7. Slope and critical shear stress evolution, for sediment supply increases (which correspond to Fig. 6 models) and decreases by factors of 5. As in figure 6,  $t=0$  corresponds to an equilibrium condition where the initial slope and initial threshold are consistent with the initial upstream sediment supply. Slope and  $\tau_c^*$  were averaged over model nodes 3-10, leaving out the first and last two nodes because of minor model boundary effects.

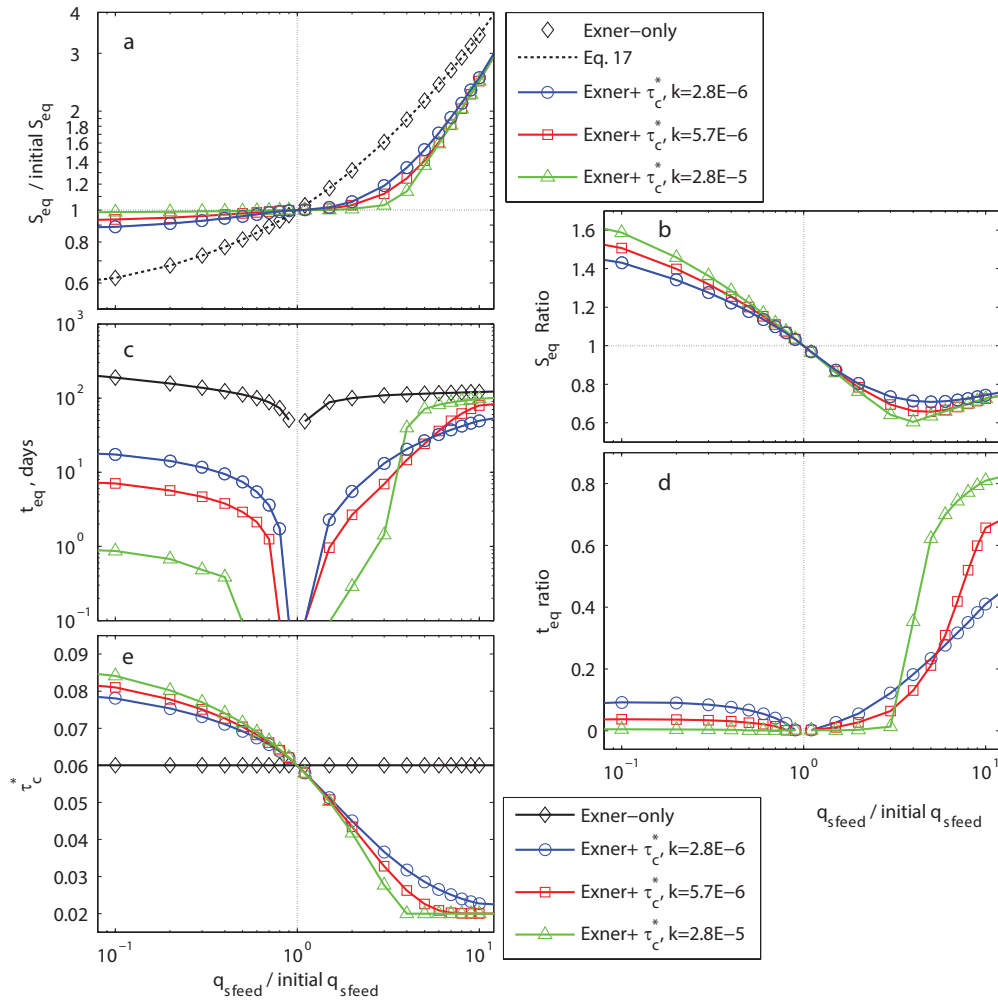


Figure 8. Morphodynamic model sensitivity to sediment supply perturbations and  $k$ . All models started at the same equilibrium condition as shown in Fig. 6 and 7. a. Slope adjustment, normalized by the initial equilibrium slope. The correspondence of Eq. 17 and the morphodynamic model calculations demonstrate that the models did asymptotically attain equilibrium slopes. b.  $S_{eq}$  ratio is the ratio of equilibrium slopes of the Exner+ $\tau_c^*$  model divided by  $S_{eq}$  for the Exner-only model, to show the relative affect that that  $\tau_c^*$  evolution has on equilibrium slopes. c. Equilibrium timescales for model adjustment. d.  $t_{eq}$  ratio is the ratio of  $t_{eq}$  for the Exner+ $\tau_c^*$  model divided by  $t_{eq}$  for the Exner-only model. Values are lower than 1, indicating that the  $\tau_c^*$  evolution has a large influence on equilibrium timescales. e. Evolution of  $\tau_c^*$ .

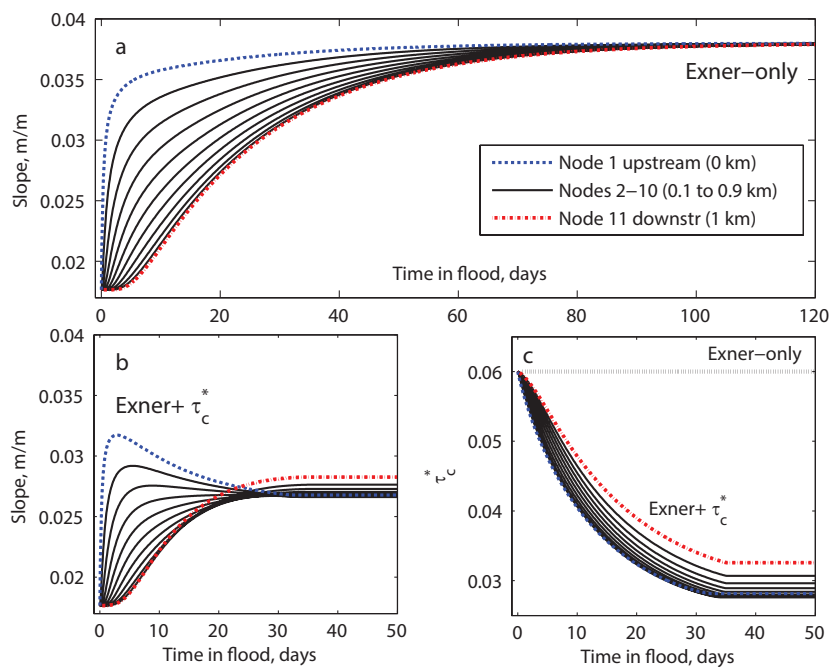


Figure 9. Spatial and temporal evolution of morphodynamic slopes, for the same models shown in Fig. 6. Slope is initially at equilibrium and responds to the 5x increase in upstream sediment supply at  $t=0$ . a. The Exner-only model initially has spatial slope variability, but evolves to a uniform new equilibrium slope. b, c. In the model with evolving  $\tau_c^*$ , slope and  $\tau_c^*$  variability persist even at equilibrium.

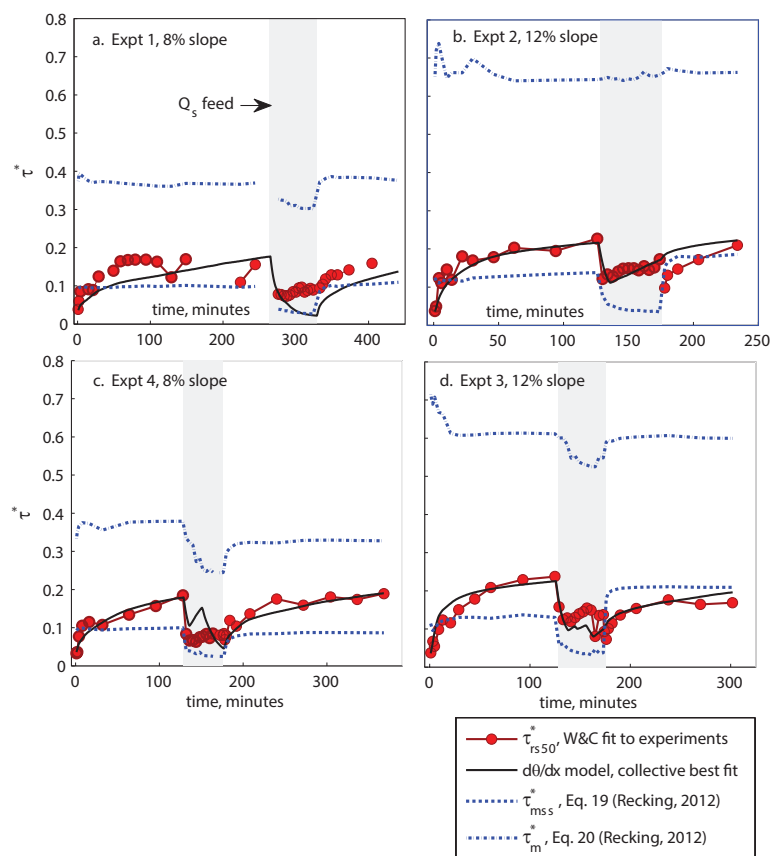


Figure 10. Comparison of experimental and best-fit model constraints on  $\tau^*_{rs50}$  compared to proposed constraints for  $D_{84}$  reference stress bounds for low and high sediment supply from Recking (2012).



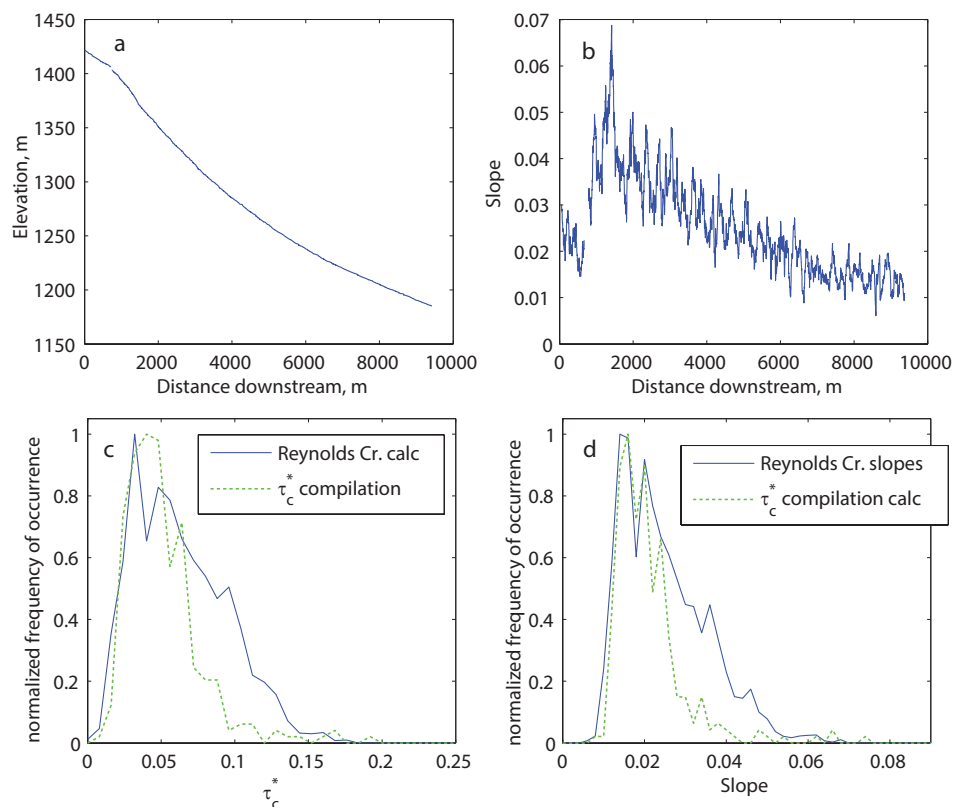


Figure 11. Comparison of Reynolds creek and  $\tau_c^*$  data compilation predictions using Eq. (18) and (21). a. Longitudinal profile based on airborne Lidar (Olinde, 2015). Upstream end (distance=0) is an arbitrary location along the channel (a bridge). Data gap at 730 m is a gauging station weir; the slope steepens downstream of the weir where the valley becomes constricted by bedrock, although the bed remains almost entirely alluvial. b. Slopes averaged over 100 m reaches, illustrating reach-scale slope variability. c. Histogram of  $\tau_c^*$  compilation (data shown in Fig. 1), compared to  $\tau_c^*$  calculated using Eq. (21), based on Reynolds Creek 100 m slopes (panel b), and assuming  $q_w=1$  m<sup>2</sup>/s,  $q_s^*=2e-3$ , and  $f=0.1$ . d. Similar calculation as panel c, but using Eq. 18 to solve for slope as a function of  $\tau_c^*$  from the Fig. 1 data compilation, and compared to panel b slopes.