# **Response to reviews on "Computing water flow through complex landscapes, part 1: Incorporating depressions in flow routing using FlowFill"**

Callaghan Kerry L.<sup>1</sup> and Wickert Andrew D.<sup>1,2</sup>

<sup>1</sup>Department of Earth Sciences, University of Minnesota, Minneapolis, MN, USA <sup>2</sup>Saint Anthony Falls Laboratory, University of Minnesota, Minneapolis, MN, USA *Correspondence to:* Kerry Callaghan (calla350@umn.edu)

#### 1 Author response

We thank the two referees, Dr Stuart Grieve and Dr Wolfgang Schwanghart, for their constructive comments on our work. I will address firstly the comments from the first referee, Dr Grieve.

- 5 **Comment**: It is unclear as to why this algorithm was not tested on higher resolution data, if there is a valid reason for this it should be communicated to the reader, otherwise, I suggest using some higher resolution data for the evaluation which is more typical of the work being done with high resolution topographic datasets in our community.
- Response: We are happy to include some results and a brief discussion of the use of this algorithm on higher-resolution (2.5
  ft) data in the final submission. We will provide an example using a subset of the Sangamon River site at this higher resolution. It does, of course, make sense to do so in light of the data that are available today and our specific discussion of the importance of distinguishing real depressions using high-resolution data. There were two reasons that we did not include this in our initial submission:
- We are very aware of the limitations of our method in terms of computational efficiency. Processing times for high resolution data will be long and while we think that the motivations for this work are particularly important where high quality,
   high resolution data are available, we also recognise that a more efficient method for dealing with the problem is needed in such cases. However, FlowFill does provide an intuitive way to visualise and map flow routing and ponding across real landscapes. The importance of highlighting the issue of real-world depressions outweighed the computational difficulties in our own method of dealing with it.
- 20 2) I decided it was more important to highlight the applicability of this method in morphologically different terrains i.e. high versus low average slope, many versus few natural depressions than to focus on a smaller, higher resolution study area. However, there is no reason not to include the high resolution data in our final submission.

**Comment**: There is no detail provided on the resampling method employed for either of the datasets. Depending on the technique used this could introduce noise into the topographic data, which may cause bias in the results by either creating new depressions or removing existing ones. I am assuming that the original data was also in gridded form, and so this effectively means that the data you are using has been resampled twice. I would like to see a clear justification of this double resampling and also some confirmation that it is not biasing the results.

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**Response**: We resampled each DEM by taking the mean value of the high-resolution cells within each low-resolution, larger cell. The reason for doing so was to produce sample study DEMs that were appropriate for our purpose of providing examples of the type of results that FlowFill can produce, in an amount of time that we found acceptable using the hardware at our disposal

10 disposal.

Resampling of data may cause bias in the results, and this is something that has been studied by other authors (e.g. Dixon and Earls, 2009). This is an issue at any time when resampling is performed, for any DEM processing method, flow accumulation, or hydrologic analysis performed. Coarsening the DEM generally results in a decrease in the total number of depressions present, since many of the smallest depressions are eliminated (MacMillan et al, 2011). Since our intention here is to provide a

15 proof of concept, rather than to specifically guide work at the locations we have studied, any bias that may have been introduced was not detrimental to our results.

While I have not yet tested this, it seems likely that using FlowFill for the same study area at different spatial resolutions will produce slightly different results. As mentioned above, we will include an example using a portion of the Sangamon River site at a higher resolution in the final submission; we will also include a comparison between results obtained at the two resolutions used. The selection of an appropriate resolution and whether or not resampling is acceptable will depend upon the user's needs and discretion.

Comment: It is not clear to me how easy it will be to use the output of this code in existing topographic analysis packages. If we do not use a maximum runoff depth, some depressions will be preserved in the data, which will cause many popular flow
routing algorithms to fail (eg Braun and Willett, 2013). If I am correct in this assumption, some more detail in the manuscript drawing out this distinction would be helpful. I think the power of this technique comes not as a replacement to existing hydrologic correction, but as a tool to identify connectivity under different runoff regimes. For example, would it be possible to couple this code with a rainfall generator such as STORM (Singer et al., 2018) to develop better understanding of landscape connectivity and surface runoff under variable rainfall conditions?

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**Response**: It is true that some flow -routing algorithms will fail once they detect depressions still in the data. However, this behavior is algorithmic instead of realistic, and a main point of our work is to demonstrate the importance of considering natural depressions in the landscape along with times in which these will (or will not) overflow.

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The referee's comment about the difference between being a replacement for existing hydrologic corrections or a tool to identify connectivity in fact identifies both sides of the same question. Existing flow-routing algorithms include an assumption of integrated drainage that may or may not be true. Our goal here is to take a step towards bridging that gap.

To directly answer the question about pre-processing: a user would have to select a flow-routing method that allows depressions to be present. For example, when creating figures for this paper, I (Callaghan) used the FlowAccumulation method from RichDEM, which worked as expected. A GRASS GIS user could use r.watershed, which allows inclusion of a layer showing which depressions on the landscape are real. For our revised manuscript, we will create a GRASS extension for FlowFill, which would make it more accessible to users.

With reference to coupling with a rainfall generator, the simplest way to use these together would be to use generated rainfall
as the 'runoff' input into FlowFill. It is in principle possible to use FlowFill with variable runoff inputs, and this functionality will become available in the next release of FlowFill. A user will be able to use an input constructed from any model or real-world data they prefer, or they can elect to use the single-value runoff we have used in our examples.

Comment: There is no discussion of the suitability of a D8 flow routing scheme as a representation of real runoff behavior.
15 Doing a full analysis of how differing schemes (D-infinity, MFD may impact the results would be well beyond the scope of this contribution, but some discussion around these issues would be a valuable addition. I think this is particularly important as the basis of this algorithm is that it is physically-based and so it is important to justify the abstraction of using D8 rather than a more "correct" routing scheme.

- **Response**: The conceptual basis of our method moving water downslope from cell to cell in such a way that the maximum amount of water moved makes the two cells equal in total elevation requires a D8 or D4 environment. Using something like D-infinity, which may move water to more than one cell at a single step, would require some reconceptualization of the technique. Any approach using D-infinity or MFD would have to be significantly different than the one used in FlowFill.
- **Comment**: Page 6, Line 3 What about integer DEM datasets? In that case ties could be very common, is there a better way to address this?

Response: While we believe that it would in general be best to avoid integer datasets in cases where more precise data is available, some users may want to use integer datasets. Even then, ties should still be less common than one may expect, since
the algorithm includes the height of the topography plus water thickness when selecting a direction. If a constant amount of water is applied across the landscape, ties would be more common at first, but they should be significantly reduced after the first few iterations. Given the possibility that ties may present a problem in some cases, the next release of FlowFill will include an option to treat ties by selecting a random direction rather than a preferential direction. Since the use of randomness will make the results non-deterministic, the user will choose which method they prefer at runtime.

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**Comment:** Page 14, Line 3 - I am asking a lot here, but is there any way to differentiate between true and spurious depressions?

**Response:** Unfortunately, that is asking a lot, which is a big reason why this problem still exists. It appears that the best way to do so is by ground inspection (Lindsay & Creed, 2006) which is often impossible. Other methods, such as modelling 5 approaches, require assumptions about what makes a depression spurious (for example, small size). If we do assume that spurious depressions are small in size (few grouped cells having erroneous data, or depressions being smaller than the error margin of the data), then FlowFill should fill these with even small amounts of runoff.

In some cases, spurious depressions may be large, for example when a ravine is dammed by a single spuriously high cell that creates an artificial depression behind it. FlowFill will not deal well with such cases, but nor will other flood-fill methods. 10 In such a case, a breaching or carving algorithm would do best, but again would ignore true depressions. I recommend a combination of FlowFill and a knowledge-based approach to correcting bad data. For example, when preparing my own data for these examples, I noticed that a man-made bridge in the Sangamon site caused water to dam behind it. Since in reality water would flow under the bridge, I manually lowered these cells by cropping them out and extrapolating data across the small gap.

15 Smaller specific line and figure comments have been dealt with as suggested, and we thank Dr. Grieve for his diligence.

Comments from the second referee, Dr Schwanghart:

**Comment:** The drawback of this [Flowfill] approach, however, is that the filling of a particular sink is highly dependent on what happens upstream. An insignificant sink along a river may not be filled, because there are sinks upstream that hold water 20 back. Sink removal is thus highly dependent on the topology of the network of topographic sinks. Of course, this may be an issue if sinks are rather a data artefact than true sinks. But DEM preprocessing techniques actually deal with and correct for these artefacts. If FlowFill is not designed for this, and line 1 on page 18 actually suggests this, than FlowFill is not really a DEM preprocessing technique. Rather, it is a highly simplified simulation tool that models water flow across landscapes 25 without loss through infiltration or evaporation.

**Response**: I agree in part with this comment: FlowFill is a simplified simulation tool that models water flow across landscapes. It has several potential uses, one of which is filling depressions in a landscape under a certain amount of runoff, and is the focus of this paper. I also agree that using this method, sink removal is highly dependent on network topology and that issues may arise in cases where sinks are data artefacts. This is an issue inherent to the method: by assuming that all depressions

in the DEM are true landscape features, we ignore cases where depressions really are data errors. Even if spurious depressions are filled by FlowFill, they have 'used up' some of the water available on the landscape and this water will not be available

further downstream.

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This is a limitation of FlowFill, but does not negate its usefulness for this application.

First of all, spurious depressions are likely to be filled with even low runoff values since they are often a result of small uncertainties (Lindsay & Creed, 2006). Secondly, differentiating real depressions from erroneous ones is challenging. Lindsay and Creed (2006) discuss a few ways of doing so. The most reliable method is ground inspection, but this is often impractical. Other methods, e.g. modelling, require assumptions to be made about what constitutes a real depression. These methods may

5 also be flagging a certain depression as real when it is not, or vice versa. However, it is important to note here that there is nothing stopping a user from first using a different method to remove depressions that they believe to be spurious, and then using FlowFill to see how hydrologic connectivity may change under different amounts of starting runoff. A user may also choose to use the output of FlowFill in more complex ways than we have here: for example, if they prefer carving to remove depressions, they may run FlowFill to determine which of the depressions should be removed from their study area, and then

10 use a carving algorithm that allows them to preserve they depressions that they wish to keep (those that FlowFill did not fill). Other depression-filling algorithms, including the flood-fill, carving, and combined methods we have discussed, consider all depressions to be spurious. FlowFill represents the other end-member: a method that considers all depressions to be real. A perfect method would be able to reliably differentiate between the two; perfect data would only contain real depressions. Since we have neither, we inherently assume that FlowFill successfully fills spurious depressions while recognising that this may not

15 always be true and that a user should be vigilant for any obvious errors.

**Comment**: I wonder whether the objective of determining the amount of water in each topographic sink given a runoff volume can be obtained more easily.

- **Response**: It certainly can at least, more easily with respect to a user's computational time. This is something that we are still working on and hope to share with the community in parts 2 and 3 of our series. The method we are working on should be much more computationally efficient, but is significantly more complex to create and explain. Despite the fact that we are working toward a better way of dealing with this particular problem, we still thought it important to publish this discussion of FlowFill for a number of reasons:
- 1) It is important to highlight this issue in DEM preprocessing and move forward discussion about it. We hope that others will also think more about this issue and potential solutions.

2) While FlowFill is not computationally efficient, it is an intuitive method that was our first step in dealing with this particular problem.

- 3) There are cases (in other applications) where FlowFill may be more useful than methods of the type you have suggested.
  For example, since FlowFill actually routes water across the land surface, it can be used to see how flow directions and water distribution on a landscape change through time. This was not the main focus of our discussion here and is closer to what you have mentioned as FlowFill being a simplified simulation tool that models water flow. FlowFill can be used as a cellular automaton with a variety of possibilities. A user can view outputs after a given amount of iterations, or at multiple intervals after a set number of iterations each time. This could provide a simplified simulation of how flood water moves across the
- 35 landscape through time. A user could also view outputs from adjacent iterations to see how dominant flow directions change

through time. These or other cell-based applications would not be possible with approaches that develop flow topology based on depressions, such as the approach proposed by the reviewer.

We will mention these alternative applications in the final version of the paper.

5 **Comment**: I think that the paper would benefit from placing more emphasis of FlowFill being a modelling tool to study hydrological connectivity of complex landscapes, rather than a preprocessing technique.

**Response**: Thank you for your comment: our intention was to focus on one specific application of FlowFill. This ties back to our answer to the question above, where we have mentioned some other possible applications of this tool. We will mention

10 these in the final version of the paper, although we do think that the preprocessing application of FlowFill is a significant one.

Smaller specific line and figure comments have been dealt with as suggested, and we thank Dr. Schwanghart for his careful attention.

# 2 Changes to the manuscript

- 15 Major changes:
  - Ran FlowFill on a small region at a variety of resolutions. Included new a table and figures comparing these resolutions, and a discussion of the impact of resampling on the results.
  - Added a discussion of usefulness of FlowFill in simulation of water flow.

# Minor changes:

- Added an explanation of the new option to randomise flow direction in the case of ties, as opposed to using a preferential direction.
  - Added a mention of the new GRASS extension, r.flowfill.
  - Mentioned the removal of man-made structures from initial topography.
  - Mentioned the possibility of using FlowFill results in conjunction with carving or breaching.
- 25 Changes to clarify figure captions.

# **Computing water flow through complex landscapes, <b>part Part 1: Incorporating depressions in flow routing using FlowFill**

Kerry L. Callaghan<sup>1</sup> and Andrew D. Wickert<sup>1,2</sup>

<sup>1</sup>Department of Earth Sciences, University of Minnesota, Minneapolis, MN, USA <sup>2</sup>Saint Anthony Falls Laboratory, University of Minnesota, Minneapolis, MN, USA

Correspondence to: Kerry Callaghan (calla350@umn.edu)

**Abstract.** Calculating flow routing across a landscape is a routine process in geomorphology, hydrology, planetary science, and soil and water conservation. Flow-routing calculations often require a preprocessing step to remove depressions from a DEM to create a 'flow-routing surface' that can host a continuous, integrated drainage network. However, real landscapes contain natural depressions that trap water. These are an important part of the hydrologic system, and should be represented in

- 5 flow-routing surfaces. Historically, depressions (or 'pits') in DEMs have been viewed as data errors, but the rapid expansion of high-resolution, high-precision DEM coverage increases the likelihood that depressions are real-world features. To address this longstanding problem of emerging significance, we developed FlowFill, an algorithm that routes a prescribed amount of runoff across the surface in order to flood depressions, but only if enough water is available. This mass-conserving approach typically floods smaller depressions and those in wet areas, integrating drainage across them, while permitting internal drainage and
- 10 disruptions to hydrologic connectivity. We present results from two sample study areas to which we apply a range of uniform initial runoff depths and report the resulting filled and unfilled depressions, the drainage network structure, and the required compute time. Typical FlowFill calculations take minutes to perform and permit more realistic analyses of water flow across landscapesFor the reach- to watershed-scale examples that we ran, FlowFill compute times ranged from approximately 1 to 30 minutes, with compute times per cell of 0.0001 to 0.006 s.

#### 15 1 Introduction

Flow routing based on Digital Elevation Models (DEMs) determines the paths taken by surface water (absent human interventions) and its associated sediment and/or dissolved load. Flow routing algorithms are applied across a broad range of fields, including hydrologic and geomorphic modeling, topographic analysis, planetary science, and palaeoclimate. They are a critical component of both hydrologic (?Ng et al., 2018; ?) and geomorphic (??) (Neal et al., 2011; Ng et al., 2018; Ray et al., 2016) and

20 geomorphic (Adams et al., 2017; Coulthard et al., 2013) models, with the former including watershed-scale processes and flood risk. In the latter case, flow routing is often recomputed over time to simulate the feedback between evolving topography and drainage patterns (Hobley et al., 2017; ?)(Hobley et al., 2017; Tucker et al., 2011). Flow-routing calculations and drainage-network construction also form the basis for topographic analysis algorithms to automatically pick channel heads (Clubb et al., 2014; Passalacqua et al., 2010; Pelletier, 2013), segment watersheds into representative hydrological units (Czuba and Foufoula-Georgiou, 2014; 1)

and link river-channel form with rates of tectonic uplift (???) or subsidence (??)(Duvall et al., 2004; Perron and Royden, 2013; Willgoose e subsidence (Paola et al., 1992; Wickert and Schildgen, 2019). These same tools have been applied to understand valley networks on Mars (Luo and Stepinski, 2009; Molloy and Stepinski, 2007), the impacts of freshwater forcing on climate during the most recent deglaciation (Ivanovic et al., 2017, 2018; Riddick et al., 2018), and links between palaeo-drainage networks

5 and modern economic and agricultural resources (Craddock et al., 2010). Flow-routing algorithms are thus vital to our understanding of the landscape, the climate landscapes, climates, and water resources.

Multiple methods exist to distribute flow across the landscape, and these range from simple approaches to find the path of steepest descent from one DEM pixel to another (?) (O'Callaghan and Mark, 1984) to full shallow-water equations solvers (?) equation solvers (McGuire et al., 2013). Simple topographically-driven flow routing approaches are the most popular be-

- 10 cause they are quick to compute (e.g., ??)(e.g., Braun and Willett, 2013; Gallant and Wilson, 1996; Schwanghart and Scherler, 2014), agnostic to the amount of rainfall or runoff applied, and applicable over length scales from puddles (e.g., ?) (e.g., Chu et al., 2013) and small catchments (e.g., Ng et al., 2018; ?) (e.g., Ng et al., 2018; Teng et al., 2017) to continents (e.g., Coe, 2000; Riddick et al., 2018; Wickert, 2016). It is this type of flow-routing algorithm that we consider for the remainder of this paper.
- Because these simple algorithms route flow down the topographic gradient, local enclosed depressions in the landscape
  present a problem. Flow cannot be routed across them, so they disconnect the hydrologic network. Prior to computing flow routing, a DEM may be preprocessed to create a flow-routing surface in which depressions are managed in order to reliably extract stream networks (Metz et al., 2011). This step often results in the removal of all depressions from the original DEM (e.g., ?????)(e.g., Jenson and Domingue, 1988; Martz and Garbrecht, 1998, 1999; O'Callaghan and Mark, 1984; Soille, 2004; Cordonni Removing depressions produces a topographic surface over which a flow-routing calculation will produce a fully connected
- 20 hydrologic network.

The outright removal of all depressions, enforcing integrated drainage, means that we are biasing our landscape or hydrologic analyses towards what we are more easily able to calculate: an integrated drainage network within a continuous downhill-sloping topography. By building such a flow-routing surface, we selectively remove information about the complexity of the real landscape. Real hydrologic networks include both transport of water across the land surface and temporary storage of

25 water in depressions.

We have developed a tool, FlowFill (?)(Callaghan, 2019), that permits flow-routing calculations across landscapes that may contain real depressions. To do so, FlowFill employs mass-conserving and hydrologically consistent depression filling, allowing a user-selected depth of runoff to be spread across the landscape and flood only those depressions that would be filled by an overland flow event of the chosen magnitude. This approach eschews the assumption of fully-integrated drainage and can

30 help to improve the fit between the computed hydrologic network and field conditions (Coe, 2000).

# 2 Background and Motivations

Most existing approaches to managing depressions in flow-routing calculations assume that these are data errors and should be removed (?)(Lindsay and Creed, 2005). In a historical context, this was a reasonable assumption: many DEMs were con-

structed from sparse data, and small data errors may have been enough to produce depressions, especially in flat areas (?)(O'Callaghan and Mark, 1984). By filling these depressions, researchers bypassed this technical challenge to begin analysing the structure of drainage networks, and continue to make new discoveries based on this approach (e.g., ??)(e.g., Hooshyar et al., 2017; Seyb However, all of these analyses rely an assumption of full drainage connectivity. This assumption may be broken in regions of

- 5 lakes and basins, such as formerly-glaciated terrains (Lai and Anders, 2018) and regions in which tectonic deformation isolates individual internally drained basins between ranges (e.g. ??)(e.g. Ballato et al., 2017; Sobel et al., 2003). Such depressions occur on a sub-continental scale as well, especially in arid regions, and our lack of an approach to self-consistently include these in our flow-routing algorithms inhibits efforts to construct and analyse large-scale drainage patterns and their changes over time (e.g., Wickert, 2016).
- Several methods have been developed to remove depressions from a DEM during creation of a flow-routing surface, or to otherwise connect drainage across the landscape. A popular choice is a flood-fill algorithm, in which the elevation of all cells in depressions are raised to the level of their outlets (e.g., ??). ? (e.g., Jenson and Domingue, 1988; Martz and Jong, 1988). Martz and Garbrecht (1998) presented an alternative approach to the flood fill, which they called 'breaching'. The breaching approach lowers select cells at depression outlets, reducing the amount by which depression cells need to be raised.
- 15 (?) Soille et al. (2003) extended this concept with their carving method. Rather than raising cells inside depressions, surrounding cells are lowered in order to eliminate all depressions in the topography. Combined methods both raise and lower cell elevations to minimize the topographic difference between the original DEM and the flow-routing surface (??). ? (Lindsay and Creed, 2005; Sch Grimaldi et al. (2007) proposed a physically-based method based on steady-state topography that adjusts the elevation of cells in a DEM to that of a continuous river long profile. Metz et al. (2011) sidestep the need for topographic adjustments by in-
- 20 stead using a least-cost-path method to determine drainage paths; this allows water to flow uphill to escape depressions and is employed in the GRASS GIS 'r.watershed' algorithm (?)(Neteler et al., 2012).

Each of the methods mentioned above ignores all depressions in the DEM, either by removing them or by allowing water to flow across them, and discounts the significant hydrologic impact of real depressions. In addition to helping to control drainage pathways (?)(Govers et al., 2000), depressions set the volume of water that can pond on the land surface

- 25 (??)(Abedini et al., 2006; Hansen et al., 1999), thereby enhancing infiltration and both slowing and reducing surface runoff (?)(Darboux and Huang, 2005). For example, in the prairie wetland region of North America, natural depressions hydrologi-cally disconnect a landscape unless a runoff event is large enough to fill and overtop them (??)(Arnold, 2010; Shaw et al., 2012). Furthermore, the growing availability of high-resolution and high-precision topographic data makes it increasingly difficult to support the assumption that these depressions are errors that should be removed (???)(Arnold, 2010; Li et al., 2011; Lindsay and Creed, 20
- 30 Even coarse-resolution data on a global scale can resolve real internally-drained basins (e.g. Riddick et al., 2018; Wickert, 2016). Thus, the degree to which the drainage is integrated and the flows are connected is not static, but rather varies as a function of runoff. All of these arguments motivate an approach that allows depressions to be filled in a way that is hydrologically realistic.

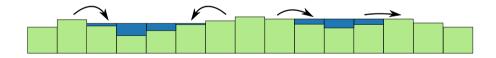
The aim of FlowFill is to generate flow-routing surfaces with an amount of drainage integration (and hence, hydrologic connectivity) that is appropriate for the amount of input runoff and the shape of the land surface. Here, we use 'drainage integration' to refer to the degree to which streams are connected (via lakes) instead of terminating in depressions, and greater drainage integration leads to a greater degree of surface-water hydrologic connectivity. Our goal in generating these surfaces is not a new one: ? Martz and Garbrecht (1998) noted the then-unrealized importance of incorporating depression storage into derived drainage patterns. ? and ? Appels et al. (2011) and Chu et al. (2013) investigated the role of microtopography in connecting

5 small surface depressions (puddles), and ?-Shaw et al. (2013) required that ponds be filled by rainfall in their contributing areas before they be allowed to spill over their boundaries and integrate into the remainder of the catchment. In our approach, we developed a cell-by-cell runoff-routing algorithm that fills depressions while conserving runoff volume in real landscapes. This open-source algorithm, FlowFill (?)(Callaghan, 2019), can compute flow-routing surfaces across a wide range of landscapes, and is applicable at a range of length scales (see Table 1).

#### 10 3 Methods

We present an algorithm to create more realistic flow-routing surfaces by flooding depressions with mass-conserved surface runoff. Depressions that are small or have large catchments become completely filled, allowing flow to cross them, whereas larger depressions may be only partially filled and continue to be hydrologic sinks (Figure 1). FlowFill works by applying a user-selected runoff depth across the landscape and moving water downslope. If a parcel of water encounters a depression,

15 as much of that parcel that can be contained by the depression before it overflows into an adjoining pixel is left behind (?)(Callaghan, 2019). This enables users to perform flow-routing across a landscape whose level of hydrologic connectivity changes through time due to storms, seasonality, or changing climate. The process of creating this flow-routing surface is summarised in Figure 2.



**Figure 1.** Filled depressions allow runoff to pass over them while unfilled depressions act as sinks to flow. The green blocks indicate the topographic surface with variable elevation, and the blue indicates the final water depth after running FlowFill. The lake on the left is not completely filled, and its level therefore is lower than the height of cells on either side. Water would continue to flow into this depression from all directions. On the right, water has completely filled a depression. Any flow entering this area from the left is able to flow out on the right and continue downslope.

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FlowFill produces a flow-routing surface through an unconditionally stable method that iteratively routes water from cell to cell across the domain (Figure 3). The required inputs are a DEM, a user-selected starting runoff value, and a user-selected threshold for convergence. This threshold specifies the maximum amount of water that may be moved between cells between two adjacent iterations for that iteration to count towards the eventual completion of the FlowFill calculation.

In Flowfill, water moves downslope, moving water from each cell in the domain once per iteration. The downstream direction for water movement is defined as the steepest downslope direction using a D8 approach (i.e. through comparison between the elevation of a 'target cell' and the elevation of the eight neighbours with which it shares either an edge or a corner). In cases in which two or more directions tie for being the steepest downslope direction, the user selects whether a preferential or a

- 5 random direction is preferred. We provide this choice since selection of a preferential direction may systematically impact the ultimate destination of the water, whereas a random direction will solve this problem to some extent but make the result non-deterministic. When a preferential direction is selected, we arbitrarily route water preferentially Northwest, then West, Southwest, South, and continue anti-clockwise with the least preferred direction being North. These cases should be rare since elevations between the cells would have to be identical to several decimals. This process of moving water down the slope of the
- 10 (topography + water) surface is repeated until a predefined criterion is met to indicate that the solution has converged: either a maximum number of iterations have been performed, or the maximum amount of water moved per iteration,  $h_{max}$ , has not changed by more than the user-defined threshold for 20,000 iterations (*i*) (Figure 4). This threshold defines the maximum value of  $|\Delta h_{max}|$  that will not reset the 'exit\_counter' that terminates the FlowFill calculation (Figure 2), where:

$$\left|\Delta h_{max}\right| = \left|h_{max}(i) - h_{max}(i-1)\right| \tag{1}$$

15 FlowFill flowchart. ' $h_{max}$ ' is the maximum amount of water moved from a single cell to another during each iteration ('*i*'). The majority of the runtime is spent sorting the cells into a priority queue and moving water from the highest cell to those below.

Once the iterative downslope movement of water has been completed, FlowFill ensures that lake surfaces are flat. Due to the iterative algorithm in FlowFill, small spurious depressions can remain following convergence. To correct for these, we search
for any pits (cells with no downslope neighbours) in the preliminary result. If a pit has one or more neighbours that contain water, it should ultimately have received water from that neighbour had FlowFill been allowed to run for longer (which is computationally expensive), so we raise its water level to the level of the water-containing neighbour. Strictly speaking, this correction means that water mass has not been conserved. However, the change affected by this correction is small relative to the total water volume. The adjustment of the lake-level surface is thresholded to a maximum value at 1/10000 of the supplied

25 runoff. The total volume of this adjustment ranged from 0 to 0.009% of the total water stored on the landscape at our study sites.

Following this lake correction, FlowFill outputs three files. The first of two binary (32-bit floating point) files contains the flow-routing surface: topography with depressions filled or partially filled in accordance with the provided input runoff depth. The second contains only the depth of water that is lying on the landscape. The third file contains runtime messages in ASCII

30 text format.

We implement FlowFill in MPI-enabled Fortran 90. This speeds calculations by splitting the domain into multiple horizontal bands with fringes that interact via the D8 flow-routing algorithm. Source code and compilation instructions are available at https://github.com/KCallaghan/FlowFill (?)(Callaghan, 2019). Use is simplified through a provided text file for users to enter parameters, and a runfile. As an additional option, users can run FlowFill through a GRASS GIS extension, r.flowfill.

The gradual cell-to-cell water redistribution within FlowFill, along with its asymptotic approach towards equilibrium due to its moving at most half of the head difference per iteration, can cause depressions to become 'overfilled' when the watermoving algorithm (Figures 3 and 2) terminates. This could happen when water has not been able to fully equilibrate over a depression, for example, when there is only a small path for water to escape a large area. These cases are not corrected inside

- 5 FlowFill, but can be corrected in an additional step through comparison with the outputs of a flood-fill algorithm applied to the same initial DEM. Flood-fill algorithms fill all depressions fully to the level of their outlets, so we correct for overfilling by (1) performing a flood fill using RichDEM's complete-depression-filling command (???)(Barnes et al., 2014a, b; Barnes, 2016), and then (2) taking the minimum of the flood fill and the FlowFill outputs to produce the final result. This correction violates conservation of water volume, but the size of the adjustment is very small relative to the total volume of water stored on the
- 10 landscape (See Section 4).

Flooded depressions have flat surfaces which can be problematic for flow routing, so flat areas were corrected using Rich-DEM to impose a gradient on these (??)(Barnes et al., 2014a; Barnes, 2016). The result is a completed flow-routing surface that retains depressions based on conservation of water volume. In order to test FlowFill, we ran it with variable initial runoff depths on two landscapes. We then routed surface-water flow over the computed flow-routing surfaces and evaluated the degree

15 of drainage integration at these locations.

## 4 Implementation

#### 4.1 Example Data

We generated flow-routing surfaces using FlowFill from DEMs of two study regions. The first study region includes a reach of the Sangamon River in Illinois, USA, located at 39.97°N, 88.72°W. The low-relief plains left behind in this post-glacial

- 20 landscape contain closed depressions that may impact hydrologic connectivity as a function of runoff (Lai and Anders, 2018). We resampled the 2.5 ft (0.76 m) resolution LiDAR DEM to 15 m resolution for our analysis. At several locations, bridges and other man-made structures cross the river channel at this study site. These are clearly visible on the lidar and artificially elevate the topography, creating blockages to water flow. These were manually removed using GRASS GIS before running FlowFill by digitising the problematic bridges, converting these to null cells, and then performing a bilinear interpolation to populate
- 25 these cells with more realistic values. The second study region was the Río Toro basin, located mainly in Salta Province, Argentina, around 24.5°S, 65.8°W. Its steep topography was shaped primarily by fluvial processes, but has also been impacted by mountain glaciation, landslide dams, and tectonically-driven isolation of much of the basin from the foreland (though it has since re-incised) (???)(Sobel et al., 2003; Tofelde et al., 2017; Trauth and Strecker, 1999). The DEM of this region was resampled to 120 m resolution from 12-m TanDEM-X data. The two landscapes differ in their topographic setting in terms of
- 30 tectonics, glaciation, drainage integration, average slope, and spatial scale.

In these examples, we prescribed uniform runoff across each DEM in order to easily compare depression filling and hydrologic connectivity (i.e. drainage integration). We tested runoff inputs of 1 em mm to 15 m in the Río Toro basin and 1 mm to 20 cm in the Sangamon River basin (Table 2). For comparison, a typical storm event in the Río Toro basin drops  $\sim$ 1 cm

#### Table 1. Characteristics of the two study sites.

connectivity in the Sangamon River basin depends on storm intensity.

large depressions persisted as we dramatically increased the initial runoff depth.

	Sangamon	Río Toro
Number of cells	298200	638154
Cell side length	15 m	120 m
High point	224.413 m	5970.257 m
Low point	186.775 m	1258.525 m
Average slope	0.0239	0.301994
Number of cells that are part of a depression	20790	3128
Depression volume per unit area	0.01437 m	0.05495 m

of rain (?, supplement)(Castino et al., 2017, supplement), which equals the minimum runoff value testeddiscussed here. This minimum amount of runoff was not capable of filling many of the depressions in the landscape, and therefore our calculations indicate that some significant segmentation of the Río Toro catchment remains regardless of the size of the rainfall event. The median daily rainfall on a day with rain near the Sangamon River is 3.3 mm, and the maximum recorded single-day rainfall is 99 cm (USGS 05590050: data from 01 October 2005 to 19 February 2019). This large range suggests that hydrologic

# 4.2 Results

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We used FlowFill to fill depressions both at the Sangamon River reach (Figures 5 and 6) and in the Río Toro basin (Figures 7 and 8). We applied varying amounts of runoff to demonstrate differing levels of depression filling (Figure 9) and hydrologic

10 connectivity (Figure 10). Both study sites contain persistent depressions that are unlikely to be permanently filled and connected via surface water, as well as smaller depressions that may be filled during modest rainfall–runoff events.

We varied the input runoff depth at the two sites, with a maximum value selected based on how much runoff was required to fill all depressions in each DEM, giving a result comparable to existing flood-fill algorithms. This maximum runoff depth was 0.2 m for the Sangamon River site (Figure 5), significantly lower than the 15 m runoff required for the Río Toro site (Figure 7). However, at the Río Toro site, most depressions were filled by significantly shallower runoff (0.1 m or less), and only a few

At both sites, deeper runoff fills more depressions, thus increasing hydrologic connectivity across the landscape (Figure 9). We define both drainage integration and hydrologic connectivity based on Strahler stream order (Figure 10 and Table 4),

which we calculated using the 'r.stream.order' extension to GRASS GIS (?)(Jasiewicz and Metz, 2011). The effect of variable runoff on drainage integration is more prominent in the Sangamon River landscape, due to its larger proportion of depressions

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runoff on drainage integration is more prominent in the Sangamon River landscape, due to its larger proportion of depressions that can, if unfilled, significantly break up the drainage network. The deep runoff required to completely fill all depressions (Figure 9) or produce higher-order drainage networks (Figure 10) implies that either heavy rainfall or a long period of rainfall – which may or may not be plausible, depending on the climate and the hydraulic conductivity of the substrate – is necessary

**Table 2.** Runtimes for mass-conserving depression filling using FlowFill. Runtimes increase with the depth of applied runoff and on flatter landscapes (Table 1).

Runoff depth [m]	Sangamon [min]	Río Toro [min]
15	-	8.55
10	-	7.15
5	_	5.23
2	_	3.8
1	_	2.88
0.5	_	2.55
0.2	32.42	2.15
0.1	28.57	1.4
0.08	29.75	_
0.05	26.62	_
0.03	24.37	_
0.02	20.07	_
0.01	14.4	1.75
0.005	2.15	_
0.001	0.97	1.63

for the landscape to become fully hydrologically connected. Both landscapes contain a few larger depressions that persist once most other depressions have been filled, though based on their locations, they have somewhat less importance in setting overall hydrologic connectivity, which saturates at runoff values below the maximum required to flood all depressions (Figure 10 and Table 4).

The processing time for FlowFill varies depending on the selected starting runoff depth, the number of cells in the domain, and the topographic structure of the site. Runtimes for our test cases varied from 0.97 minutes to 32.42 minutes (Table 2). We performed each calculation on using 8 processor cores of processors on an Intel i7-5820K CPU (3.30 GHz) on a desktop computer running Ubuntu Linux with 64 GB DDR3 RAM and a solid-state hard drive.

Due to the slight overfilling of some depressions in the outputs from FlowFill, a correction was performed, as discussed in
Section 3. In the two sample study regions discussed in this paper, the volume of the adjustment for overfilling was insignificant, ranging from 0.003% to 0.29% of the total volume of water stored on the landscape. Cases in which the supplied initial runoff was deeper tended to have a slightly higher proportion of overfilling.

# 5 Discussion

In addition to these two study sites, we used FlowFill on a subset of the Sangamon study site at four different resolutions in order to assess how the resolution of the input data affects the results. The results of this analysis can be seen in Figures 12 and 13. A small subset of the Sangamon study site was selected in order to keep runtimes manageable at higher resolutions. The resolutions selected were 0.762 m (2.5 ft, the original resolution at which we obtained this data), 3 m, 5 m, and 15 m to match

5 the resolution used for the entire Sangamon study site.

FlowFill is able to create more realistic The results show that resampling data to a different resolution has an impact both on the number and morpology of depressions in the unfilled DEMs, and on the results obtained from FlowFill. When resampling to coarser resolutions at this site, the number of large depressions in the study area visually appears to increase, as seen in Figure 12. However, Table 3 shows us that the total number of depressions actually drastically decreases. This is due to

- 10 the abundance of small depressions in the higher resolution data. Instead, the total area of depressions present increases in lower-resolution data: the total area of depressions present in the unfilled DEM at 15 m resolution is almost 70% more than the area of depressions at 0.762 m resolution. This trend is also reflected in the intermediate 3 m and 5 m resolutions. While these coarser resolutions have resulted in higher depression areas, the smoothing effect of resampling has also resulted in depressions becoming shallower, and hence, total depression volumes are smaller at coarser resolutions. This effect is less
- 15 consistent: 15 m resolution exhibits the lowest total depression volume and 0.762 m has the highest, but the 5 m resolution DEM has a higher depression volume than the 3 m resolution DEM. Systematic changes in the shape of the landscape occur as resolution changes, but not all of these changes relate linearly to the change in resolution. All of these differences in depression number and morphology at different resolutions are an important reminder that the results of any landscape study based on remotely sensed data such as this are limited by the accuracy of the input data, and any pre-processing steps may change this
- accuracy. The effects of different resolutions on depression storage are discussed in more detail by Abedini et al. (2006), and Dixon and Earls (2009) discuss the effects of different resolutions on watershed delineation and streamflow prediction.
   We completed 12 runs of FlowFill, using DEMs with each the four resolutions, and with starting runoff depths of 0.2 m, 0.01 m, and 0.005 m (Figure 12 and 13, and Table 3). Regardless of the resolution, 0.2 m starting runoff filled all of the

depressions, while lower amounts of runoff left some depressions unfilled. Overall patterns in depressions filled appear visually

- 25 similar at all resolutions, with 15 m resolution showing the greatest difference from other resolutions. At 15 m resolution with 0.01 m runoff, several of the larger depressions on the southern edge have been filled, while these remained unfilled at finer resolutions. Drainage patterns also follow similar patterns at different resolutions, with the dominant river channels visible at all four resolutions. Channel widths are inflated at coarser resolutions due to the larger cell size (Figure 13). Flow-routing pathways also differ between the 15 m resolution DEMs and those at finer resolution. When depressions are fully filled in
- 30 the finer-resolution DEMs, a channel in the northeast corner of the DEM flows off the southern edge of the map. At 15 m resolution, however, the head of this channel is diverted, and flows off the eastern edge of the map.

Using data at the highest available resolution prevents loss or distortion of data and ensures that analyses are not introducing any additional errors due to downsampling. Unfortunately, runtimes for FlowFill for large data sets can become prohibitively long. This is a limitation of FlowFill, and more computationally efficient methods for dealing with depressions in flow-routing

35 surfaces by accounting are needed. We begin to address this problem in a companion paper (Barnes et al., 2019).

Runtimes for this subset of the Sangamon site (Table 3) ranged from 2 seconds to 13 minutes for 15 m, 5 m, and 3 m resolutions with all input runoff depths, but escalated drastically to over 33 hours for 0.762 m resolution with 20 cm initial water depth. The reason for this nonlinearity lies in the fact that FlowFill moves water from cell to cell (Figure 3). Increasing the resolution both increases the total number of cells that must be calculated and requires more iterations of cell-to-cell water water for the vector to move the same real world distance. But times times are non-cell or given in Figure 11.

5 exchange for the water to move the same real-world distance. Runtimes times per cell are given in Figure 11.

# 5 Discussion

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The flow-routing surfaces created by FlowFill account for water stored in the landscape and disconnects in the drainage network. The importance of such an approach is apparent because depressions persist in flow-routing surfaces even when the prescribed initial runoff is deep. This indicates that preprocessing topographic data with algorithms that fill all depressions is likely to result in spuriously integrated drainage networks. We have demonstrated that this effect can occur in both high-

- and low-relief landscapes, and that in addition to correcting spurious depressions, true lake basins and swales must be taken into account. Spurious depressions can also be filled by runoff and will therefore also be corrected by FlowFill. Some of the available runoff will be used up in doing so. Because spurious depressions caused by data errors are likely to be small, these would be filled with even low amounts of runoff (??)(Lindsay and Creed, 2006; O'Callaghan and Mark, 1984), though it is
- 15 still possible that some depressions which that remain unfilled are artefacts of data errors.

FlowFill provides users with a completed flow-routing surface, however, should a user prefer carving, breaching, or combined methods for depression removal, these can still be used in conjunction with FlowFill. The result obtained from FlowFill determines which depressions should be removed during creation of the flow-routing surface, and which should remain. Depressions that FlowFill has not completely filled can be masked out, while those which were completely filled can be

20 selectively carved or breached. This allows a user to utilise their preferred depression removal method, while still being cognisant of the importance of retaining real-world depressions.

# 5.1 Hydrologic connectivity

We present FlowFill results at two locations with differing landscape characteristics (Table 1). The results include flow-routing surfaces with selectively filled depressions and the associated changes in drainage integration of the landscapes. A visual

- 25 inspection of stream networks in cases where lower runoff values are used tells us that hydrologic connectivity is lower in these cases. This is quantitatively supported by the Strahler stream order data shown in Table 4. More higher-order streams occur when deeper runoff is used to create the flow-routing surfaces, hence filling more depressions. Channels were extracted using the FlowAccumulation functionality in RichDEM (Barnes et al., 2014a; Barnes, 2016) with a threshold of 100 units of accumulation.
- 30 The impact of using different amounts of runoff within FlowFill on the hydrologic connectivity of the landscape was more apparent in the Sangamon River basin study site, where more depressions were present. The presence of these depressions significantly reduced connectivity between stream segments. It is likely that FlowFill will be most useful in cases where the

geologic and geomorphic history of a landscape produce a surface with many depressions, such as this post-glacial landscape. The high number of depressions seen may also be partially due to the finer resolution of this data relative to the Río Toro study site – density of depressions has been shown to relate to grid spacing as an inverse power law (?) (Lindsay and Creed, 2005) – but this does not belie the finding that significant real depressions exist and impact hydrologic connectivity.

- The ability to use FlowFill with varying user-selected starting runoff values makes it ideal for comparing network connectivity in wet versus dry seasons (Figure 10 and Table 4), or for analysing the effects of storms of different sizes. Shallow runoff inputs to FlowFill imitate real-world conditions with low amounts of rainfall (e.g. during the dry season). During these times, hydrologic connectivity is significantly reduced, and routing flow across a completely depression-filled landscape becomes unrealistic. Deep runoff inputs simulate wet seasons or flood conditions. Due to the associated greater hydrologic connectivity,
- 10 more of the region contributes water to basin outlets.

# 5.2 Cellular-based modeling

FlowFill is a cellular automaton that models water flow across landscapes. While we designed FlowFill to fill depressions on digital elevation models in a way that conserves water mass, rather than to reproduce a physics-based transient flow response, we propose that its mechanism of moving flow between cells may be useful for modelling applications. The amount of water

- 15 moved between cells at each iteration is gradient-based, meaning that FlowFill can approximate real transient flow in the landscape as a result of both topography (body forces) and water depth (pressure forces). Where the outputs of FlowFill diverges significantly from a true flow solution, for example, to the backwater equation, is that a parcel of water in FlowFill can move at most one cell per iteration, regardless of the underlying slope. Furthermore, FlowFill cannot accommodate different roughness values that would modulate flow velocity in one region versus another. With these limitations in mind,
- it could still provide a useful approach for simulations with approximately constant roughness and in which differences in elevation are consistently less than the flow depth. Fortunately, such examples are common in geomorphology, and include reduced-complexity approaches towards simulating the dynamics of braided rivers (Murray and Paola, 1997) and river deltas (Liang et al., 2015b, a). With these uses in mind, users can view intermediate (pre-equilibrium) result outputs from FlowFill, after a set number of iterations or at frequent intervals. The compute time for an individual iteration of FlowFill ranged from 10<sup>-3</sup> to 10<sup>-6</sup> s for the regions discussed in this article.

# 5.3 Limitations

While we have created a way to handle the problem of real-world depressions in a DEM, FlowFill does have some limitations. Firstly, FlowFill requires significantly more compute time than flood-fill methods that fully fill DEM sinks (???)(Barnes et al., 2014a; Barnes Secondly, it creates flat areas in DEMs, which present their own challenges for flow routing. Thirdly, the threshold value for

30  $|\Delta h_{max}|$  is distinct in different landscapes, and as such is a user-defined parameter. Finally, if an input topography contains three-dimensional structures such as bridges, FlowFill can cause water to artificially dam behind them.

FlowFill can be time-consuming to run, especially for large DEMs, high starting runoff depths, or relatively flat study sites. Runtimes for each of the results shown here are shown in Table 2, with the compute time per cell shown graphically in Figure 11. This may make it an unappealing choice in cases with large study areas or very high-resolution data. Therefore, while FlowFill can route runoff to create flow-routing surfaces, a more computationally efficient solution to the problem outlined in this paper will permit faster analyses of a wider range of DEMs.

Like some other depression-filling algorithms, FlowFill produces flat areas where it fills depressions. Post-processing these into a gentle slope may be required in order to create reasonable or visually appealling flow networks. Fortunately, tools to do so efficiently already exist. To produce our drainage networks for the stream-order calculations (Figure 10), we used RichDEM to impose a gradient on flat areas (??) (Barnes et al., 2014a; Barnes, 2016) as the final step in constructing each flow-routing surface.

While a single criterion for convergence on a final flow-routing surface would be ideal, we were unable to find one, and

- 10 instead have left this as a user-selected parameter. We attempted to use the maximum amount of water moved between two cells in an iteration, the total amount of water moved in an iteration, the rate of change in the maximum amount of water moved between two cells in an iteration (averaged over various time windows), and the root-mean-square error of the linear regression that created the aforementioned slope. We also attempted each of these methods while normalizing for the initial amount of applied runoff. None of these approaches were able to collapse the response curves of flow over the landscape. However, we
- 15 do observe that the maximum amount of flow between two cells in a single iteration asymptotes to a consistent value in each landscape. We therefore selected the exit criteria based on a time when the change in the maximum amount of flow between cells from one iteration to the next is below some small, user-selected threshold. It is necessary for a user to select this threshold value since the amount of noise after the plateau has been reached varies from one landscape to the next. As a result, we suggest that users who want to test multiple initial runoff depths first run FlowFill with a modest amount of runoff (to speed compute
- 20 time: Figure 11) in order to create a maximum water depth moved vs. iteration curve like those shown in Figure 4. From this, users may pick a threshold value for  $|\Delta h_{max}|$ . Selecting a value that is too large will cause FlowFill to exit before reaching this plateau, while a value that is too small will cause FlowFill to continue running for its maximum limit of 1,000,000 iterations, resulting in a long compute time. Based on our two study areas, suitable threshold amounts are landscape-specific, but appear to be agnostic to the amount of runoff selected for a given landscape. Following this approach, we chose thresholds of 0.01
- 25 mm (Sangamon) and 1 mm (Río Toro).

All depression-filling algorithms can produce 'lakes' as artefacts where the two-dimensional topography does not represent efficient three-dimensional flow paths – such as flow under bridges or through culverts (Lindsay and Dhun, 2015; Passalacqua et al., 2012). FlowFill is especially sensitive to these, as their damming effect can also create bottlenecks that significantly increase the number of iterations required to evacuate the water behind them, even when a narrow flow path exists to by-

30 pass them. Even after convergence, these areas often require the additional step to reduce overfilling, discussed in Section 3. This common problem further motivates work to remove these artificial blockages from rivers in DEMs for flow routing (?)(Abdullah et al., 2012).

#### 6 Conclusions

Common and efficient downslope flow-routing algorithms must be run across surfaces that properly represent a true surfacewater-potential surface. As modern DEM resolution and accuracy increases, this requires that DEM depressions be appropriately filled. We have developed an algorithm, FlowFill, that fills only those depressions on a landscape which would become

5 filled under reasonable runoff conditions. This allows for the existence of real depressions and hydrologic disconnects in the landscape.

By adding more realistic surface-water hydrology to flow routing, FlowFill's ability goes beyond that of static flood-fill algorithms and enables scientists to examine dynamic hydrologic connectivity. While FlowFill effectively solves this important problem, its long runtimes for larger datasets can make its use inconvenient. Future advances towards a more computationally

10 efficient methodology will aid in the longer-term goal of linking real-world data with algorithms that harness the emerging power of far-reaching and high-resolution topographic data.

*Code availability.* The latest version of FlowFill is available at https://github.com/KCallaghan/FlowFill. The most recent version at the time of publication, 1.1.0-beta (Callaghan, 2019), is archived at https://doi.org/10.5281/zenodo.2586244. The GRASS extension r.flowfill is available at https://github.com/OSGeo/grass-addons/tree/master/grass7/raster/r.flowfill.

15 *Author contributions.* KLC developed the algorithm behind FlowFill, wrote its source code, ran the tests, and was the primary author of the manuscript. ADW provided guidance in the parallelization, suggested the test sites, and co-wrote the manuscript.

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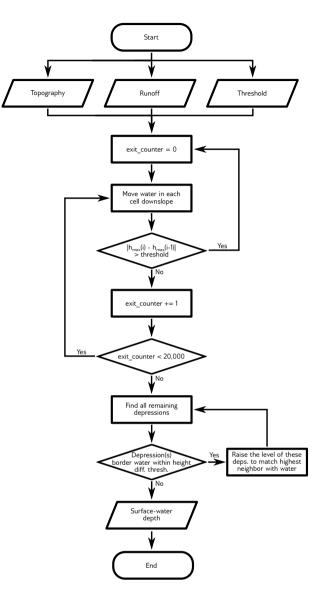
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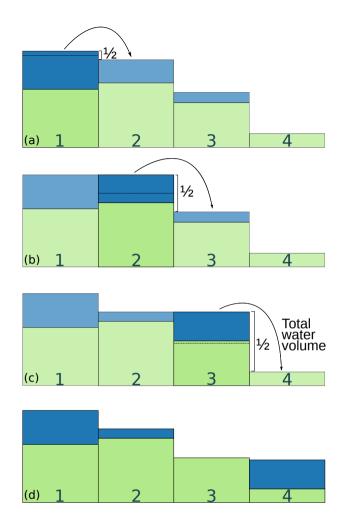
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**Figure 2.** Water flow during a single iteration of FlowFill flowchart. In each iteration, water is moved starting with the highest (water + topography) cell and ending with the lowest. Each 'target cell $h_{mex}$ ' routes water into its steepest downslope neighbour. The is the maximum amount of water moved from the higher a single cell to the lower one is the the minimum of either all the water available in the target cell or half the difference in elevation between the target cell and its steepest downslope neighbour. This latter criterion ensures numerical stability, but slows convergence towards a solution. (a) Starting at the highest another during each iteration (topography + water depth'i) cell, half of the difference in topographic + water thickness height is moved from cell 1 to cell 2, this being the steepest downslope direction. (b) Cell 2 becomes the target cell. Half The majority of the difference between cells 2 and 3 is moved to cell 3. (c) Cell 3 becomes the target cell. There runtime is less water available in cell 3 than half the difference between cells 3 and 4, so all of the spent moving water from cell 3 is moved to cell 4. (c) Cell 4 becomes the cach target cell ; if this were the edge of the domain, the water in cell 4 would flow out of the domain. (d) A single iteration has been completed. Water will now start moving from cell 1 again, and the process will repeat until the solution converges to those below.



**Figure 3.** Water flow during a single iteration of FlowFill. In each iteration, water is moved starting with the highest (water + topography) cell and ending with the lowest. Each 'target cell' routes water into its steepest downslope neighbour. The amount of water moved from the higher cell to the lower one is the minimum of either all the water available in the target cell or half the difference in elevation between the target cell and its steepest downslope neighbour. This latter criterion ensures numerical stability, but slows convergence towards a solution. (a) Starting at the highest (topography + water depth) cell, half of the difference in topographic + water thickness height is moved from cell 1 to cell 2, this being the steepest downslope direction. (b) Cell 2 becomes the target cell. Half of the difference between cells 2 and 3 is moved to cell 3. (c) Cell 3 becomes the target cell. There is less water available in cell 3 than half the difference between cells 3 and 4, so all of the water from cell 3 is moved to cell 4. (d) Cell 4 becomes the target cell; if this were the edge of the domain, the water in cell 4 would flow out of the domain. A single iteration has been completed. Water will now start moving from cell 1 again, and the process will repeat until the solution converges.

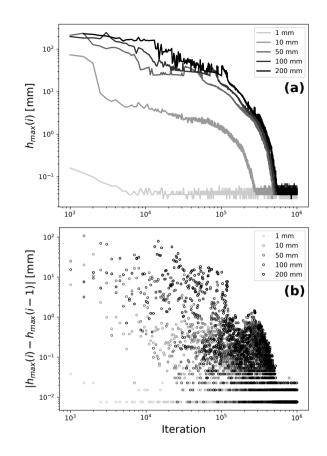
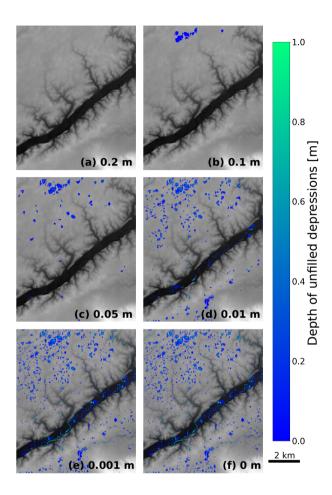
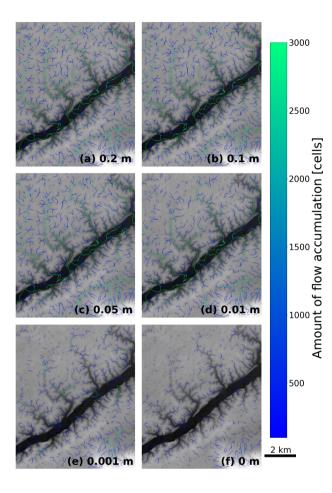


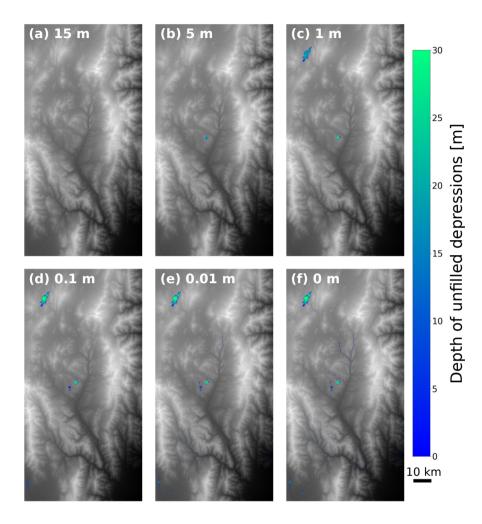
Figure 4. The maximum amount of water moving from one cell to another in a single iteration  $(h_{max})$  is useful in deciding when to threshold FlowFill's result. Most water moves near the beginning of a model run. Panel (a) shows how  $h_{max}$  changes for 5 different model runs (light grey to black, with depths of 1–200 mm given in the legend) at the Sangamon River site. Each run continues for 1,000,000 iterations (*i*) and each shows a distinct plateau at which  $h_{max}$  ceases to consistently decrease. Prior to this plateau, we see some fluctuations in the amount of water moving per iteration as some water makes it to river channels and some depressions become filled, changing the evolving flow-routing surface. False plateaus represent periods of time in which the maximum amount of water moving per iteration does not significantly change. In order to avoid exiting the program early during one of these false plateaus, we conservatively wait for a plateau that lasts 20,000 iterations before thresholding our result. Panel (b) shows  $|\Delta h_{max}|$  (Equation 1), the absolute value of the change in  $h_{max}$  between the current and the previous iteration. Based on these data, we were able to select the threshold for this site as  $|\Delta h_{max}| = 0.01$  mm. Once  $|\Delta h_{max}| < 0.01$  for 20000 iterations, the model run saves the result and is complete.



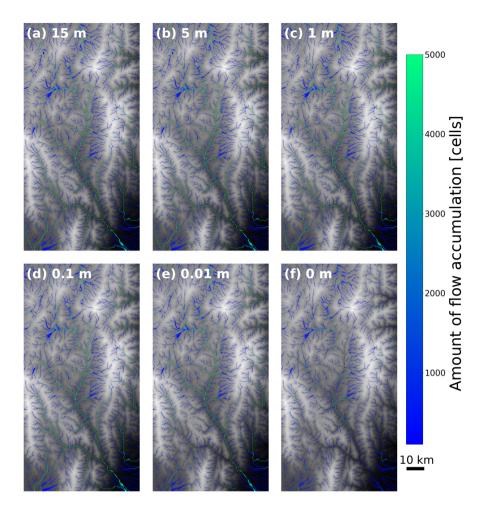
**Figure 5.** Depressions remaining with different amounts of starting runoff at the Sangamon River site. Deeper runoff fills more depressions. Unfilled depressions are shown for varying initial runoff depths: (a) 0.2 m, (b) 0.1 m, (c) 0.05 m, (d) 0.01 m, (e) 0.001 m, (f) 0 m (i.e. the input DEM with no changes made). DEM elevations are represented by a dark (low) to light (high) greyscale, while blue and green colours indicate the depths of depressions still present in the flow-routing surface. In the case of 0.001 m runoff, many depressions still remain, while with 0.1 m of starting runoff all but the largest depressions are filled. Depressions were fully filled with a starting runoff depth of 0.2 m.



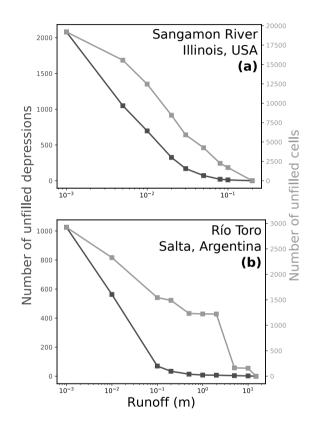
**Figure 6.** Drainage networks on partially-filled landscapes at the Sangamon River site. Flow networks were created using the FlowAccumulation method included in RichDEM (Barnes et al., 2014a; Barnes, 2016) with the result masked to show only cells with a flow accumulation greater than 100. Hydrologic connectivity changes depending on how many depressions remain in the landscape. The unfilled depressions associated with each panel are shown in Figure 5. The colours indicate the amount of flow accumulation in stream channels. Higher runoff depths applied to FlowFill fill more depressions and result in higher degrees of hydrologic connectivity. In panel (**a**), with 0.2 m of runoff, all depressions were filled: drainage is fully integrated, and the result is identical to that for a flow-routing surface created using standard flood-fill techniques (e.g., ??)(e.g., Barnes et al., 2014a; Barnes, 2016). In panels (**b**)–(**e**), decreasing amounts of starting runoff result in increasing segmentation of the stream network. Panel (**f**) shows the original DEM, which hosts only a few disconnected stream segments.



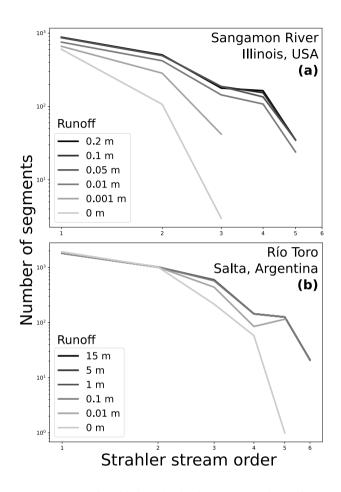
**Figure 7.** Depths of unfilled depressions in the Río Toro study area with starting runoff depths of (**a**) 15 m, (**b**) 5 m, (**c**) 1 m, (**d**) 0.1 m, (**e**) 0.01 m, and (**f**) 0 m (i.e. the input DEM with no changes made). DEM elevations are shown in greyscale, from dark (low) to light (high), while blue and green colours indicate the locations of depressions still present in the flow-routing surface. (**a**) In the case where 15 m of runoff was used, all depressions in the DEM were filled. (**b**) With 5 m of runoff, we see a persistent depression near the centre of the study region. (**c**) Another large depression appears at with 1 m of runoff, but most smaller depressions are filled. (**d**,**e**) With 0.1 m runoff and less, more depressions appear in the landscape. (**f**) All of the depressions appear on the original, unfilled DEM.



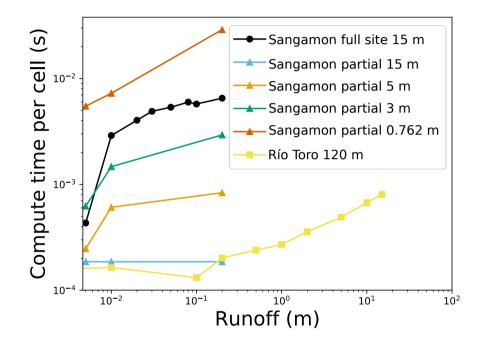
**Figure 8.** Drainage networks on partially-filled landscapes at the Río Toro site. Flow networks were created using the FlowAccumulation method included in RichDEM (Barnes et al., 2014a; Barnes, 2016) with the result masked to show only cells with a flow accumulation greater than 100. Hydrologic connectivity changes depending on how many depressions remain in the landscape. The unfilled depressions associated with each panel are shown in Figure 7. The colours indicate the amount of flow accumulation in stream channels. Higher runoff depths applied to FlowFill fill more depressions and increase hydrologic connectivity. (a) With 15 m of runoff, all depressions were filled so the result is identical to a flow-routing surface created with other flood fill techniques. The drainage is fully integrated. In panels (b) and (c), with 5 m and 1 m of runoff depth respectively, hydrologic connectivity changes only slightly. The depressions that appear with these amounts of runoff are near the headwaters of the river network, making the changes in hydrologic connectivity in these cases minimal. In panels (d) and (e), reduced runoff starts to create more disconnects in the stream network appear. Panel (f) shows the original DEM, which has the lowest degree of hydrologic connectivity. Strahler stream orders associated with each panel are shown in Table 4.



**Figure 9.** Number of cells (light grey) and depressions (dark grey) that remain unfilled under different starting runoff depths in (**a**) the Sangamon River basin and (**b**) the Río Toro basin sites. Similar trends are seen at both study sites, with higher runoff resulting in more depressions being completely filled. See supplementary information for exact figures.



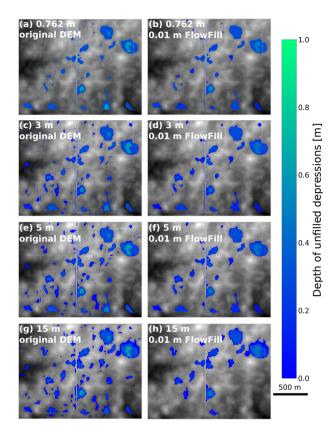
**Figure 10.** <u>Strahler stream orders</u>. As more water is added to the landscape, more depressions are flooded and drainage integration (and therefore hydrologic connectivity) increases. More stream segments overall exist on the landscape, and they become more connected, increasing the fraction of higher-order streams. Lines representing channel networks with shallower runoff depths overlie those representing deeper runoff for (**a**) the Sangamon River site and (**b**) the Río Toro site. We computed the stream orders in GRASS GIS (Neteler et al., 2012) using r.watershed (Metz et al., 2011) to compute stream networks, followed by r.stream.order to calculate the Strahler stream orders (Strahler, 1957).



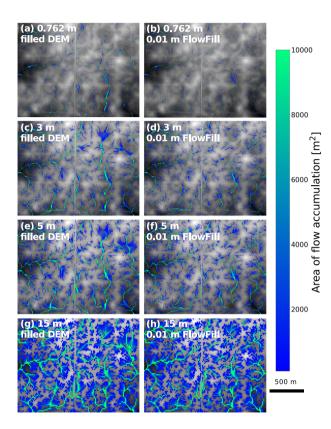
**Figure 11.** Time taken, in seconds per cell, for FlowFill to run to completion with different depths of starting runoff. Runs using larger starting runoff values take longer, and runs on a flatter spatial domain take longer. The Sangamon River DEM contained 298200 cells with total runtimes ranging from 59 to 1945 seconds. The Río Toro DEM contained 638154 cells with total runtimes ranging from 98 to 513 seconds. Details on cell counts and runtimes for the partial section of the Sangamon site are listed in 3.

**Table 3.** FlowFill runs on a subset of the Sangamon study site at four different resolutions and with varying amounts of starting runoff. Runtimes increase with the depth of applied runoff, and with increasing number of cells in the domain. Compute times per cell can be seen in Figure 11. The number of depressions is greater at higher resolutions: number of depressions scales linearly with the number of cells in the domain, where *number\_of\_depressions* =  $0.0034756 \times number_of\_cells + C$ . Depression areas tend to be greater at coarser resolutions.

Resolution	Number of cells	Runoff depth [m]	Runtime [min]	Number of unfilled depressions	Area of unfilled depressions [m <sup>2</sup> ]	Volume of unfilled depressions [m <sup>3</sup> ]
15	10738	0.200	0.03333	$\overset{0}{\sim}$	0.00000	0.00000
15	10738	0.010	0.03333	<u>16</u>	207675.00000	36344.89860
15	10738	0.005	0.03333	31	256950.00000	42649.75318
15	10738	0.000	≂	<u>84</u>	318150.000000	50542.00515
5	<u>96996</u>	0.200	1.35000	0	0.00000	0.00000
5	96996	0.010	0.98333	<u>45</u>	210525.00000	41696.87690
5	<u>96996</u>	0.005	0.40000	72	233275.00000	48746.75788
5	96996	0.000	-	414	283100.00000	56780.37223
3	269173	0.200	13.16667	$\overset{0}{\sim}$	0.00000	0.00000
3	269173	0.010	6.60000	112	201654.00000	40731.05525
$\frac{3}{\sim}$	269173	0.005	2.80000	218	216459.00000	47874.59637
3	269173	0.000	≂	1254	245943.00000	55914.46614
0.762	4176000	0.200	2013.71667	0	0.00000	0.00000
0.762	4176000	0.010	504.56667	3112	172913.46062	47454.86143
0.762	4176000	0.005	381.55000	<u>5091</u>	177010.48468	54796.08098
0.762	4176000	0.000	~	14649	188244.20415	63816.53063



**Figure 12.** Depths of unfilled depressions in a subsection of the Sangamon study area at several different resolutions. The left column shows depressions existing in the unfilled DEM, and the right column shows remaining depressions after running FlowFill with 1 cm of starting runoff. The panels are (a) the unfilled DEM at 0.762 m (2.5 ft) resolution, (b) the 0.762 m resolution results after running FlowFill with 1 cm starting water, (c) 3 m resolution unfilled DEM, (d) 3 m resolution after running FlowFill, (e) 5 m resolution unfilled DEM, (f) 5 m resolution after running FlowFill, (g) 15 m resolution unfilled DEM, and (h) 15 m resolution after running FlowFill. DEM elevations are shown in greyscale, from dark (low) to light (high), while blue and green colours indicate the locations of depressions still present in the flow-routing surface. Resampling to coarser resolutions creates the visual impression of increasing the number of depressions, which are lost at coarser resolutions. The results after using FlowFill appear visually similar, with the exception of the 15 m resolution DEM, in which several larger depressions along the Southern margin were filled. Table 3 reveals that hundreds to thousands of less visible, smaller depressions were filled at finer resolutions.



**Figure 13.** Drainage networks in a subsection of the Sangamon study area at several different resolutions. Flow networks were created using the FlowAccumulation method included in RichDEM (Barnes et al., 2014a; Barnes, 2016) with the result masked to show only cells with a flow accumulation greater than  $500 m^2$ . The original, unfilled DEM supported very little drainage and is not pictured here. On the left are the fully-connected drainage networks occurring over a completely filled flow-routing surface. On the right are the partially connected networks resulting from the partially-filled surfaces created using FlowFill and 1 cm of starting runoff. The unfilled depressions associated with the right-hand column are shown in Figure 12. The colours indicate the amount of flow accumulation in stream channels, in  $m^2$ . The main river channels are consistently present at all resolutions.

			Río Toro			
Stream order	15 m runoff	5 m runoff	1 m runoff	0.1 m runoff	0.01 m runoff	Original DEM
1	1816	1816	1813	1805	1859	1922
2	1014	1014	1023	1017	1016	1018
3	593	593	584	573	439	216
4	144	144	144	144	85	58
5	126	126	126	126	115	1
6	21	21	21	21	0	0
Sangamon River						
Stream order	0.2 m runoff	0.1 m runoff	0.05 m runoff	0.01 m runoff	0.001 m runoff	Original DEM
1	879	871	865	754	661	602
2	505	496	493	420	284	107
3	178	187	187	144	42	3
4	164	154	135	108	0	0
5	35	35	35	24	0	0

**Table 4.** Number of streams of each Strahler order at each study site after flow-routing surfaces were created using different amounts of runoff. Deeper initial runoff was able to fill more depressions, integrating flow across them and building higher-order drainage networks.