# <sup>1</sup> Supplementary material

- <sup>2</sup> Biogeomorphic modeling to assess
- <sup>3</sup> resilience of tidal marsh restoration to sea
- 4 level rise and sediment supply
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Figure S1: Vegetation dynamics model scenarios (i.e., reference vegetation dynamics and two variants, respectively without vegetation and with instantaneous colonization – Sect. 2.3.3). Evolution of the mean platform elevation with respect to the mean high-water level (MHWL) (a-b) and development of the vegetation cover (c-d) in the Northern (a, c) and Southern basins (b, d).



Figure S2: Vegetation dynamics model scenarios (i.e., reference vegetation dynamics (a) and two variants, respectively without vegetation (b) and with instantaneous colonization (c) – Sect. 2.3.3). Bed elevation 50 years after de-embankment. The dashed lines delineate the old marsh, the Northern basin, and the Southern basin. The ellipses emphasize a preexcavated channel that has disappeared (a-b) or survived (c), depending on the vegetation dynamics. All figures are rotated by 43° clockwise, as compared to Fig. 2c.



31

32 Figure S3: Reference model scenario (#1). Mean elevation change (between years 18 and 50 for model results, between 1931

and 1963 for observations) vs. mean high-water depth (in year 18 for model results, in 1931 for observations). Model results
 and observations are respectively split into 10 sub-samples of equal size (Sect. 2.4.4). Markers and error bars represent the
 means and standard deviations of each sub-sample. Dashed lines represent linear regressions of the sub-sample means.

36 Model results are on grid nodes that remained vegetated between years 18 and 50. Observations are from areas that remain

vegetated between years 1931 and 1963 in an established marsh nearby the study site (Sect. 2.4.1) and have been rescaled

38 to account for differences in SSC in both sites (Sect. S2).



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40 Figure S4: Reference model scenario (#1). Development of vegetation cover after de-embankment (blue) compared to

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Figure S5: Inlet design model scenarios (i.e., reference design and three alternative designs with small-inlet breach size of respectively 50, 100 and 200 m, and excavated channel – #1, 6-8). Evolution of the mean platform elevation with respect to the mean high-water level (MHWL) (a) and development of the vegetation cover (b) in the Northern and Southern basins combined.



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Figure S6: Reference model scenario (#1). Channel geometric properties 10, 20 and 50 years after de-embankment (different shades of blue) compared to observations in an established marsh nearby the study site (black). Probability distribution of the unchanneled flow length (a), upstream mainstream length vs. watershed area (b), and channel width (c), channel depth (d) and channel cross-section area (e) vs. mean overmarsh tidal prism. (b-e) Model results and observations are respectively split into 10 sub-samples of equal size (Sect. 2.4.4). Markers and error bars represent the geometric means and standard deviations of each sub-sample. Dashed lines represent geometric regressions of the geometric means.



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Figure S7: Suspended sediment concentration model scenarios (#1, 4). Channel geometric properties 50 years after deembankment (blue, red) compared to observations in an established marsh nearby the study site (black). Probability distribution of the unchanneled flow length (a), upstream mainstream length vs. watershed area (b), and channel width (c), channel depth (d) and channel cross-section area (e) vs. mean overmarsh tidal prism. (b-e) Model results and observations are respectively split into 10 sub-samples of equal size (Sect. 2.4.4). Markers and error bars represent the geometric means

60 and standard deviations of each sub-sample. Dashed lines represent geometric regressions of the geometric means.

Table S1: Coefficients of determination ( $R^2$ ) of the linear regressions obtained from model results and observations in Fig. 4

62 and S3, and *p*-values of the ANCOVA performed to determine whether linear regressions from model results and

63 observations are statistically equal (both *p*-values must be higher than 0.05). The first *p*-value determines whether the slopes

64 of the linear regressions are significantly different (if p < 0.05) and the second p-value whether their intercepts are

65 significantly different (if p < 0.05).

Figure	$R^2$ (model)	$R^2$ (observations)	p (slopes)	p (intercepts)
Figure 4b	0.985	0.977	0.913	0.007
Figure 4c	0.955	0.929	0.001	< 0.001
Figure 4d	0.973	0.929	0.056	< 0.001
Figure 4e	0.985	0.929	0.023	0.004
Figure S2	0.966	0.987	0.496	0.412

## 67 S1 Biogeomorphic model

68 We have developed the biogeomorphic modeling framework Demeter to simulate explicitly the feedbacks between hydrodynamics, morphodynamics (Sect. S1.1) and vegetation 69 70 dynamics (Sect. S1.2). This is a multiscale approach, in which the vegetation dynamics is computed at much finer resolution than the hydro-morphodynamics (Fig. 1), requiring the 71 72 development of specific multiscale coupling techniques to preserve subgrid-scale 73 heterogeneity while information is exchanged between the hydro-morphodynamic and 74 vegetation modules (Sect. S1.3 and S1.4). The specific setup for our study site is detailed in 75 Sect. S1.5.

### 76 S1.1 Telemac (hydro-morphodynamics)

As hydro-morphodynamic module (Fig. 1a), we use the finite element solver suite Telemac
(version 7.3.0), and more specifically its modules Telemac-2D for the hydrodynamics and
Sisyphe for the sediment transport and the morphodynamics.

Telemac-2d solves the depth-averaged shallow water equations in a two-dimensional horizontal framework (Hervouet, 2007) to simulate fluctuations of the water depth h and the depth-averaged flow velocity u:

$$\frac{\partial h}{\partial t} + \boldsymbol{\nabla} \cdot (h\boldsymbol{u}) = 0 \tag{S1}$$

$$\frac{\partial \boldsymbol{u}}{\partial t} + \boldsymbol{u} \cdot \boldsymbol{\nabla} \boldsymbol{u} = -g \boldsymbol{\nabla} \eta + \frac{1}{h} \boldsymbol{\nabla} \cdot (h \boldsymbol{\nu} \boldsymbol{\nabla} \boldsymbol{u}) - \frac{\boldsymbol{\tau}_{\boldsymbol{b}} + \boldsymbol{\tau}_{\boldsymbol{\nu}}}{\rho h}$$
(S2)

83 where *t* is the time,  $\nabla$  is the spatial differential operator, *g* is the gravitational acceleration,  $\eta$  is the water surface elevation above the reference level (NAP),  $\nu$  is the diffusion coefficient,  $\tau_b$  is the bed shear stress,  $\tau_v$  is the vegetation resistance force per unit horizontal area, and  $\rho$  is the water density. The bed shear stress is computed with the Manning formula:

$$\boldsymbol{\tau}_{\boldsymbol{b}} = \frac{\rho g n^2}{h^{1/3}} \|\boldsymbol{u}\| \boldsymbol{u}$$
(S3)

- where the Manning coefficient *n* is empirically derived and depends mainly on bed roughness.
  The vegetation resistance force is modeled as the drag force on a random or staggered array
  of rigid cylinders with uniform properties (Baptist et al., 2007) and depends on the spatial
  distribution of vegetation provided by the cellular automaton (Sect. S1.4).
- Sisyphe solves the depth-averaged advection-diffusion equation to simulate fluctuations
  of the depth-averaged suspended sediment concentration *C*:

$$\frac{\partial hC}{\partial t} + \boldsymbol{\nabla} \cdot (h\boldsymbol{u}C) = \boldsymbol{\nabla} \cdot (h\boldsymbol{v}\boldsymbol{\nabla}C) + E - D$$
(S4)

where *E* and *D* are the rates of sediment erosion and deposition, respectively. The rate of
 sediment erosion is computed using the equation of Partheniades (1965):

$$E = \begin{cases} M\left(\frac{\|\boldsymbol{\tau}_b\|}{\tau_e} - 1\right) & \text{if } \|\boldsymbol{\tau}_b\| > \tau_e \\ 0 & \text{otherwise} \end{cases}$$
(S5)

where *M* is the Partheniades constant and  $\tau_e$  is the critical bed shear stress for sediment erosion. The rate of sediment deposition is computed using the equation of Einstein and Krone (1962):

$$D = w_s C \tag{S6}$$

98 where  $w_s$  is the sediment settling velocity. The evolution of the bed is computed as follows:

$$\frac{\partial b}{\partial t} = \alpha \frac{D - E}{\rho_s} \tag{S7}$$

99 where *b* is the bed surface elevation above the reference level (NAP),  $\alpha$  is the morphological 100 acceleration factor (Sect. 2.1) and  $\rho_s$  is the sediment dry bulk density. The bed is composed 101 of two layers: the fresh layer at the surface and the compacted layer underneath. Their 102 evolution obeys the following rules: (i) each layer is characterized by different values of  $\tau_e$ 103 and  $\rho_s$ , (ii) erosion of the compacted layer only occurs where and when the fresh layer is 104 locally empty, (iii) deposition only occurs on the fresh layer, and (iv) there is no sediment flux 105 between the two layers.

#### **106** S1.2 Cellular automaton (vegetation dynamics)

As vegetation module, we use the cellular automaton implemented in Demeter. A cellular automaton consists of a regular grid of cells, each one with a finite number of states (here, either bare or one of the considered vegetation species). Cells can change their state in discrete time steps, depending on their neighborhood state and a set of simple stochastic transition rules (Balzter et al., 1998).

- 112 S1.2.1 Establishment
- Establishment is the transition from bare state 0 to any vegetated state *i*. The probability of establishment  $p_i^{est}$  for species *i* is evaluated as:

$$p_i^{est} = P_i^{est} \prod_k f_k \tag{S8}$$

- where  $P_i^{est}$  is the background probability of establishment for species *i*, and  $f_k$  are stress functions of the environmental variables (Sect. S1.2.5).
- 117 S1.2.2 Succession
- 118 Succession is the transition from any vegetated state i to another vegetated state j > i (e.g.,
- from pioneer to climax vegetation). The probability of succession  $p_{i,j}^{suc}$  from species *i* to *j* is evaluated as:

$$p_{i,j}^{suc} = P_{i,j}^{suc} \prod_{k} f_k \tag{S9}$$

- 121 where  $P_{i,i}^{suc}$  is the background probability of succession from species *i* to *j*.
- 122 S1.2.3 Stress-related die-off

Stress-related die-off (or simply die-off) is the transition from any vegetated state *i* to bare state 0 due to environmental stress. The probability of die-off  $p_i^{die}$  for species *i* is evaluated as follows:

$$p_i^{die} = 1 - \prod_k (1 - f_k)$$
 (S10)

#### 126 S1.2.4 Annual die-off

127 Annual die-off is the transition from any vegetated state *i* to bare state 0 due to the natural 128 cycle of annual species. The probability of annual die-off  $p_i^{ann}$  for species *i* is evaluated as 129 follows:

$$p_i^{ann} = P_i^{ann} \tag{S11}$$

- 130 where  $P_i^{ann}$  is the background probability of annual die-off for species *i*.
- 131 S1.2.5 Stress functions

Stress functions (Sect. S1.2.1 to S1.2.3) can be of two shapes. When vegetation is only affected at high (resp. low) values of an environmental stressor, and not below (resp. above) a certain threshold, we use the Hill function, which varies from 0 to 1 following:

$$f_H(x;H,N) = \frac{x^N}{H^N + x^N}$$
(S12)

where x is the environmental variable, H is the threshold around which the transition from 0 to 1 occurs, and N is a parameter that controls the shape of the function. The function decreases from 1 to 0 if N < 0 and increases from 0 to 1 if N > 0. The transition from 0 to 1 becomes steeper for increasing |N|. When the range of optimal conditions is confined between a low and a high thresholdvalue, we use the Brière function:

$$f_B(x; X_0, X_1) = \max\left(\frac{x(x - X_0)(X_1 - x)}{c}, 0\right)$$
(S13)

141 where  $X_0$  and  $Xx_1$  are the low and high thresholds, respectively, and c is a coefficient used to 142 rescale the function, so that its maximum value is 1:

$$c = x_{opt} (x_{opt} - X_0) (X_1 - x_{opt})$$
(S14)

$$x_{opt} = \frac{1}{3} \left( X_0 + X_1 \sqrt{X_1^2 - X_0 X_1 + X_0^2} \right)$$
(S15)

143 The different environmental variables used for the stress functions are the hydroperiod, 144 the bed elevation gain and loss, and the binned shear stress (Sect. S1.3.1).

#### 145 S1.2.6 Lateral expansion

Lateral expansion is the transition from any state *i* (bare or vegetated) to any vegetated state j > i resulting from the presence of at least one neighboring cell of state *j*. The recruitment process is here quite different than for the other processes. It is defined by the mean expansion rate  $R_i^{exp}$ , which determines the number of iterations  $N_{exp}$  of the cellular automaton. For each iteration, the probability of recruitment by lateral expansion  $p_i^{exp}$  is

$$p_i^{exp} = \frac{R_i^{exp}}{N_{exp}\Delta x}$$
(S16)

151 where  $\Delta x$  is the grid resolution of the cellular automaton. With this stochastic approach, even 152 though the mean expansion rate is constant, the actual expansion rate varies in space and 153 time. The number of iterations is determined so that

$$R_{max} > R_i^{exp} + 2\sigma^2 \tag{S17}$$

where the maximum expansion rate  $R_{max}$  and the variance of the expansion rate  $\sigma^2$  are calculated as follows:

$$R_{max} = N_{exp} \Delta x \tag{S18}$$

$$\sigma^{2} = N_{exp} p_{i}^{exp} \left(1 - p_{i}^{exp}\right) \Delta x = R_{i}^{exp} \left(1 - \frac{R_{i}^{exp}}{N_{exp} \Delta x}\right)$$
(S19)

As each species can have a different mean expansion rate, and hence a different number ofiterations, we use the highest number of iterations among all species.

## 158 S1.2.7 Computational sequence

159 The different transition rules of the cellular automaton are scheduled as follows:

- 160 1. Annual die-off is applied for each annual species in one single iteration.
- Establishment, succession, and lateral expansion are applied for all species in an
   iterative process. The number of iterations is determined based on the mean
   expansion rates (Sect. S1.2.6). For each iteration, the probabilities of establishment,
   succession and lateral expansion are rescaled as follows:

$$p_i^{est} \leftarrow 1 - (1 - p_i^{est})^{N_{exp}} \tag{S20}$$

$$p_{i,j}^{suc} \leftarrow 1 - \left(1 - p_{i,j}^{suc}\right)^{N_{exp}} \tag{S21}$$

$$p_i^{exp} \leftarrow p_i^{exp} \frac{N_{nb}}{4} \tag{S22}$$

- 165 where  $N_{nb}$  is the number of neighboring cells vegetated with the same species *i* at 166 the previous iteration. We use a factor  $\frac{N}{4}$  in Eq. S22, so that the rescaling factor  $\frac{N_{nb}}{4}$  is 167 1 on average.
- 168 3. Stress-related die-off is then applied in one single iteration.
- 169 S1.3 Coupling Telemac to cellular automaton
- 170 S1.3.1 Environmental variables

171 The hydroperiod  $T_H$  is the percentage of time during which a Telemac grid node is flooded 172 (i.e., the water depth higher than 0.1 m) between two cellular automaton calls. It varies 173 between 0 (never flooded) and 1 (always flooded).

174 The bed elevation change  $\Delta b$  is the difference between the final and initial bed elevations 175 between two cellular automaton calls. The bed elevation gain  $\Delta b_+$  and the bed elevation loss 176  $\Delta b_-$  are calculated as:

$$\Delta b_{+} = \max(\Delta b, 0) \tag{S23}$$

$$\Delta b_{-} = \max(-\Delta b, 0) \tag{S24}$$

The binned shear stress is calculated by classifying flow directions into 8 directional bins 177 (45° each) occurring between two cellular automaton calls. The relative binned time  $T^i$ , the 178 binned shear stress  $ar{ au}^i_b$ , and the binned water depth  $ar{h}^i$  are respectively the percentage of 179 time, the mean bed shear stress, and the mean water depth when the flow is oriented in the 180 *i*<sup>th</sup> bin. As bed shear stress and flow directions are especially relevant above certain thresholds 181 182 of the water depth and the bed shear stress, these binned variables only account for situations when the water depth is higher than 0.1 m and the bed shear stress is higher than 183 0.1 N m<sup>-2</sup>. 184

185 The mean water depth  $\overline{h}$  between two cellular automaton calls is calculated for situations 186 when the water depth is higher than 0.1 m.

187 S1.3.2 Spatial refinement

188 We use a linear interpolation to spatially refine the hydroperiod, and the bed elevation gain 189 and loss from the Telemac grid to the cellular automaton grid.

We use the concepts of Voronoi neighborhood to spatially refine the relative binned time and the binned water depth. Each cellular automaton grid cell is associated with its closest Telemac grid node. The Voronoi neighborhood of a Telemac grid node is the ensemble of all associated cellular automaton grid cells. Here, the relative binned time and the binned water depth of a Telemac grid node are passed to all cellular automaton grid cells of its Voronoi neighborhood.

For the binned shear stress, we use a convolution method that allows to account for interactions between flow and subgrid-scale vegetation patterns (Gourgue et al., 2021). Practically, we first calculate the binned velocity  $\bar{u}^i$  on the Telemac grid as follows:

$$\bar{u}^{i} = \left(\frac{\bar{\tau}_{b}^{i} (\bar{h}^{i})^{1/3}}{\rho g n^{2}}\right)^{1/2}$$
(S25)

Then, we use a convolution method (Gourgue et al., 2021) to spatially refine the mean binned
velocity from the Telemac grid to the cellular automaton grid. Finally, we calculate the binned
shear stress on the Telemac grid as follows:

$$\bar{\tau}_{b}^{i} = \frac{\rho g n^{2}}{\left(\bar{h}^{i}\right)^{1/3}} \left(\bar{u}^{i}\right)^{2}$$
(S26)

202 S1.3.3 Stress function of the binned shear stress

A stress function of the binned shear stress (typically using the Hill function) requires a specific
 treatment to combine all its components. It is calculated as follows:

$$f = 1 - \prod_{i=1}^{8} \left( 1 - f_H(\bar{\tau}_b^i; H, N) \right)^{T^i}$$
(S27)

- 205 S1.4 Coupling cellular automaton to Telemac
- 206 S1.4.1 Vegetation resistance force

For the vegetation resistance force per unit horizontal area  $\tau_v$  in Eq. S2 of the hydromorphodynamic module, we use the approach introduced by Baptist et al. (2007), which considers plants as rigid cylinders with uniform morphological properties (i.e., stem density,
diameter and height). As compared to the original method, we here neglect the extra term
depending on the ratio between water depth and plant height, and we combine linearly the
separate effect of each plant species:

$$\boldsymbol{\tau}_{\boldsymbol{\nu}} = \frac{1}{2} \rho \beta \left( \sum_{i} C_{Di} \gamma_{i} m_{i} d_{i} \min(h, k_{i}) \right) \|\boldsymbol{u}\| \boldsymbol{u}$$
(S28)

where  $\beta$  is the transmittance coefficient (Sec. S1.4.2), and  $C_{Di}$ ,  $\gamma_i$ ,  $m_i$ ,  $d_i$  and  $k_i$  are respectively the bulk drag coefficient (Baptist et al., 2007), the vegetation cover (Sec. S1.4.2), the stem density, the stem diameter and the stem height of species *i*.

#### 216 S1.4.2 Spatial coarsening

The vegetation cover  $\gamma_i$  of the species *i* is the percentage of cellular automaton cells of state *i* within the Voronoi neighborhood of a Telemac grid node (Sec. S1.3.2). It varies between 0 (not covered by species *i*) and 1 (fully covered by species *i*). The sum of all vegetation covers also varies between 0 (bare) and 1 (fully covered by vegetation).

221 The transmittance coefficient  $\beta$  accounts for the spatial heterogeneity of the vegetation 222 distribution at the subgrid scale (i.e., within a Voronoi neighborhood). In general, hydrodynamic models assume a uniform spatial distribution at the subgrid scale (here,  $\beta =$ 223 1), which leads to considerable overestimation of the flow resistance if the vegetation 224 225 presents clustered patterns at the subgrid scale (Gourgue el al, 2019). The method to 226 compute the transmittance coefficient  $\beta$  builds on the similarity between the Chézy formula 227 in fluid dynamics and Ohm's law in electricity. Taking the analogy further, we recalculate the 228 coarse-scale hydraulic roughness just as the total resistance of an electronic circuit that 229 combines resistors (equivalent to cellular automaton cells in our analogy) connected in series (along-flow) and in parallel (across-flow). The transmittance coefficient  $\beta$  is calculated at the 230 231 end of a cellular automaton call. It varies between 0 and 1 and it has different values 232 depending on the flow direction (Gourgue el al, 2019).

233 S1.5 Study site setup

#### 234 S1.5.1 Hydro-morphodynamic module

The initial bed elevation is based on the project design (Sec. 2.2 and Fig. 2) and Lidar data before de-embankment. The bed is initially exclusively composed of a compacted layer. Tides 237 are imposed into the system by defining water levels and flow velocities at the open boundary between the study site and the Scheldt Estuary, which is here approximately the isobath 5 m 238 239 below the mean low water level. These boundary conditions are provided by a 3D 240 hydrodynamic model of the estuary, which has been calibrated for a spring-neap cycle by comparison with measurements of water levels, flow velocities and water discharges 241 242 (Maximova et al., 2014). To reduce the computational time, we do not simulate the entire 243 range of tidal conditions of a full spring-neap cycle. Instead, we only select four different semi-244 diurnal tidal cycles from the estuarine model, which are representative of the standard range 245 of tidal conditions that can be observed in that area. With high water levels of 2.05, 2.55, 2.87 246 and 3.25 m NAP, the selected tidal cycles have a frequency distribution of respectively 14.6%, 247 27.4%, 32.3% and 25.7%, as compared to historical measurements during the period 2007-248 2017. These frequency distributions are then used to determine the morphological 249 acceleration factor  $\alpha$  used for each semi-diurnal tidal cycle (Sec. 2.1). We simulate the impact 250 of sea level rise by lowering the bed elevation every year by a value corresponding to the 251 yearly increase of mean sea level. The suspended sediment concentration at the open 252 boundary is constant and determined based on reported measurements (Vandenbruwaene 253 et al., 2014; Sec. S2). All parameter values used in the hydro-morphodynamic module are 254 summarized in Table S2. The suspended sediment concentration at the open boundary and 255 the rate of sea level rise vary according to model scenarios (Table 1).

256 Table S2: Hydro-morphodynamic module parameter values.

Parameter	Symbol	Value	Value Reference	
Gravitational acceleration	g	9.81 m s <sup>-2</sup>	Standard	
Diffusion coefficient	ν	1 m² s <sup>-1</sup>	Calibration	
Water density	ρ	1000 kg m <sup>-3</sup>	Standard	
Manning coefficient	n	0.021 s m <sup>-1/3</sup>	Maximova et al., 2014	
Partheniades constant	М	10 <sup>-4</sup> kg m <sup>-2</sup> s <sup>-1</sup>	D'Alpaos et al., 2012; Zhou et al., 2016	
Critical had arosion choor stross	τ <sub>e</sub>	0.5 N m <sup>-2</sup> (fresh layer)	Zhou et al., 2016	
Citical bed erosion shear stress		0.8 N m <sup>-2</sup> (compacted layer)	Zhou et al., 2016; D'Alpaos et al., 2012	
Settling velocity	W <sub>s</sub>	1 mm s <sup>-1</sup>	van Leussen, 1999	
Morphological acceleration factor	α	103 (neap tide) 193.5 (mid-neap tide) 228 (mid-spring tide) 181.5 (spring tide)	Sec. 2.1 and S1.5	
Dry bulk density	$ ho_s$	500 kg m <sup>-3</sup> (fresh layer) 1500 kg m <sup>-3</sup> (compacted layer)	Van de Broek et al., 2018	
Bulk drag coefficient	C <sub>D</sub>	2 (pioneer marsh) 1 (middle marsh) 5 (high marsh)	Calibration (Gourgue et al., 2021) with flume measurements (Schwarz et al., 2015)	
Stem density	m	214 m <sup>-2</sup> (pioneer marsh) 338 m <sup>-2</sup> (middle marsh) 298 m <sup>-2</sup> (high marsh)	Field observations	
Stem diameter	d	12mm (pioneer marsh) 6.8mm (middle marsh) 5.98mm (high marsh)	Field observations	
Stem height	k	1 m (pioneer marsh) 1.03 m (middle marsh) 2.36 m (high marsh)	Field observations	

#### 258 S1.5.2 Vegetation module

259 The study site is in the oligonaline zone (0.5 - 5 PSU) where Aster tripolium is often 260 observed as the pioneer species, and *Scirpus maritimus* and *Phragmites australis* in the marsh 261 interior (Van Braeckel et al., 2008). Their expected encroachment in our study site is further 262 supported by the results of transplantation experiments carried out in nearby tidal marshes. 263 Aster tripolium is an annual species, which can be found as lower pioneer in calm areas and 264 along creek edges. It colonizes the tidal flats and creek levees every year from seeds, as 265 randomly scattered high density clusters on tidal flats. Although it is regarded as an annual 266 species, part of the established plants can survive and develop for another year. Scirpus 267 maritimus is the dominant perennial species from the low pioneer zone into the middle marsh 268 zone. It is even the only species present in the pioneer zone in several tidal marshes close to 269 the study site. The main mode of colonization on bare tidal flats is via lateral spread of 270 rhizomes (Silinski et al., 2016). Phragmites australis is the dominant species in the high marsh 271 zone. It can form large stands from the high pioneer zone up to the supratidal zone, but it is 272 mostly found above Scirpus maritimus in the middle and high marsh zone. Most seedling 273 establishment occurs within already established vegetation, but very rarely on bare tidal flats, 274 except for the highest areas. Once established, it can often outcompete Scirpus maritimus 275 and colonize vegetated areas by lateral expansion via rhizomes, resulting in clearly visible circular patches within Scirpus maritimus marshes. 276

The initial vegetation distribution is based on aerial pictures before de-embankment. Marshes that will be excavated and farmland are considered as unvegetated. Parameterization of the different stress functions (Sec. S1.2.5) is based on field and flume experiments, remote sensing, literature data and model calibration (Tables S3-S4).

Process Contribution Reference Aster tripolium (species 1, pioneer marsh)  $P_1^{est} = 0.2$ Calibration  $f_B(T_H; 0.039, 0.1134)$ Field experiments; Silinski et al., 2016 Establishment (Eq. S8)  $f_H(\Delta b_+; 0.03 \text{ m}, -2.37)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\Delta b_-; 0.001 \text{ m}, -4)$  $f_H(T_H; 0.129, 25)$ Field experiments; Silinski et al., 2016  $f_H(T_H; 0.019, -31)$ Die-off (Eq. S10 and S27)  $f_H(\Delta b_-; 0.02 \text{ m}, 6.32)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\bar{\tau}_b^i; 0.2 \text{ N m}^{-2}, 15)$ Flume experiments; calibration  $P_1^{ann} = 0.5$ Annual die-off (Eq. S11) Calibration Scirpus maritimus (species 2, middle marsh)  $P_2^{est} = 10^{-5}$ Calibration  $f_B(T_H; 0.011, 0.105)$ Field experiments; Silinski et al., 2016 Establishment (Eq. S8)  $f_H(\Delta b_+; 0.03 \text{ m}, -2.37)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\Delta b_-; 0.001 \text{ m}, -4)$  $f_H(T_H; 0.38, 40)$ Field experiments; Silinski et al., 2016  $f_H(T_H; 0.001, -41)$ Die-off (Eq. S10 and S27)  $f_H(\Delta b_-; 0.075 \text{ m}, 4)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\bar{\tau}_b^i; 0.15 \text{ N m}^{-2}, 15)$ Flume experiments; calibration Lateral expansion  $R_2^{exp} = 2.25 \,\mathrm{m}$ Remote sensing; Silinski et al., 2016 Phragmites australis (species 3, high marsh)  $P_3^{est} = 5 \times 10^{-7}$ Calibration  $f_H(T_H; 0.035, -8.5)$ Field experiments; Silinski et al., 2016 Establishment (Eq. S8)  $f_H(\Delta b_+; 0.03 \text{ m}, -2.37)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\Delta b_-; 0.001 \text{ m}, -4)$  $P_{23}^{suc} = 2.5 \times 10^{-6}$ Calibration Succession (Eq. S9)  $f_H(T_H; 0.054, -6.5)$ Field experiments; Silinski et al., 2016 (from Scirpus maritimus)  $f_H(\Delta b_+; 0.03 \text{ m}, -2.37)$ Bouma et al., 2016; Cao et al. 2018  $f_H(\Delta b_-; 0.001 \text{ m}, -4)$  $f_H(T_H; 0.13, 20)$ Field experiments; Silinski et al., 2016  $f_H(\Delta b_-; 0.1 \text{ m}, 6.32)$ Die-off (Eq. S10 and S27) Bouma et al., 2016; Cao et al. 2018  $f_H(\bar{\tau}_b^i; 0.12 \text{ N m}^{-2}, 15)$ Flume experiments; calibration  $R_3^{exp} = 2.25 \text{ m}$ Lateral expansion Remote sensing; Silinski et al., 2016

#### Table S3: Vegetation module parameterizations (reference vegetation dynamics, used in model scenarios #1-8).

Table S4: Vegetation module parameterizations (instantaneous colonization, used in reference model scenario variant).

Process	Contribution	Reference					
Aster tripolium (species 1, pioneer marsh)							
Establishment (Eq. S8)	$p_1^{est} = \begin{cases} 1 & if \ H < 0.1134 \\ 0 & if \ H \ge 0.1134 \end{cases}$	Field americante Ciliada et al. 2010					
Die-off (Eq. S10)	$p_1^{die} = \begin{cases} 0 & if \ H < 0.129 \\ 1 & if \ H \ge 0.129 \end{cases}$	Field experiments; Silinski et al., 2016					
Scirpus maritimus (species 2, middle marsh)							
Succession (Eq. S9) (from <i>Aster tripolium</i> )	$p_{1,2}^{suc} = \begin{cases} 1 & if \ H < 0.078 \\ 0 & if \ H \ge 0.078 \end{cases}$	Field experiments; Silinski et al., 2016					
Die-off (Eq. S10)	$p_2^{die} = \begin{cases} 0 & if \ H < 0.38 \\ 1 & if \ H \ge 0.38 \end{cases}$						
Phragmites australis (species 3, high marsh)							
Succession (Eq. S9) (from <i>Scirpus maritimus</i> )	$p_{2,3}^{suc} = \begin{cases} 1 & if \ H < 0.044 \\ 0 & if \ H \ge 0.044 \end{cases}$	Field experiments; Silinski et al., 2016					
Die-off (Eq. S10)	$p_3^{die} = \begin{cases} 0 & if \ H < 0.13 \\ 1 & if \ H \ge 0.13 \end{cases}$	· · · ·					

## 283 S2 Sediment accretion on vegetated platforms

Based on digital elevation maps derived from historical topographic surveys in the adjacent marshes of the Drowned Land of Saeftinghe (Fig. 2c) between 1931 and 1963 (Wang and Temmerman, 2013), we have developed an empirical relationship between mean elevation change on vegetated platforms and mean high-water depth (Vandenbruwaene et al., 2014). Here, we develop a similar relationship based on model results in the restored tidal marsh, using the same variables over the same time interval (i.e., between years 18 and 50 after deembankment), and we compare it with the empirical relationship derived from observations.

The digital elevation maps derived from historical topographic surveys have a resolution of 20 m. To focus on vegetated platforms and avoid the influence of tidal channels, we only consider vegetated areas that are at least 200 m from tidal channels in the digital maps (Vandenbruwaene et al., 2014). Similarly, as our model results have a resolution of 5 m, we only consider areas that are at least 50 m from tidal channels in the model results. The Drowned of Saeftinghe is located downstream of the study site, where the sediment input from the Scheldt Estuary is substantially lower. Historical measurements in the period 2001-2012 reveal that the tide-averaged SSC in the estuary is 42 mg l<sup>-1</sup> close to the Drowned of Saeftinghe and 63 mg l<sup>-1</sup> close the study site (Vandenbruwaene et al., 2014). To account for this 1.5 ratio in sediment input between model and observations, we multiply the observed mean elevation change by 1.5 to obtain the data presented in Fig. S1.

## 302 S3 Pioneer vegetation development

303 We compare our model results with observed rate of spatial expansion of the vegetation 304 cover in the adjacent restored marshes of Paardenschor (Fig. 2c), from the onset of vegetation in 2007 until 2017. We use a series of Google Earth images, and we apply the method of 305 Richardson et al. (2009) to classify vegetation pixels. Part of the vegetation colonization in 306 307 Paardenschor starts from the dikes. Such phenomenon is expected to be of a much lesser influence in our study site. Hedwige-Prosper Polder is about 30 times larger than 308 Paardenschor, hence the average distance to dikes will be much higher. In our analysis, we 309 310 therefore remove the vegetation development occurring from the dikes.

## 311 S4 Channel network characteristics

312 We compare various geometric properties of the simulated tidal channels with observations 313 in the adjacent marshes of the Drowned Land of Saeftinghe (Fig. 2c – Vandenbruwaene et al., 314 2013, 2015). To that end, we have developed a quasi-automatic methodology to extract tidal channel networks and related characteristics from model results. We first identify grid nodes 315 316 within channels by applying a multi-window median neighborhood analysis (Liu et al., 2015) 317 on the simulated topography, and we compute the unchanneled flow length as the shortest distance to a channel grid node (Tucker et al., 2001). We then retrieve channel edges as 318 multiple polygons by applying the Python function tricontour from the visualization library 319 320 Matplotlib (Hunter, 2007) on the channel grid nodes. We finally extract the channel network 321 skeleton, defined as the channel centerlines (Fagherazzi et al., 1999), by generating the raw 322 Voronoi diagram of the channel edge polygons (with the Python library Centerline) and 323 applying straightforward threshold rules to simplify it.

324 We use a virtual topography method to determine the watershed areas along the network skeleton (Vandenbruwaene et al., 2013, 2015). In terrestrial river networks, watershed areas 325 326 are exclusively delineated by topographic gradients. For tidal channel networks, however, 327 topographic gradients are small and water flow is mainly determined by water surface gradients (Rinaldo et al., 1999). Alternatively, algorithms designed for terrestrial river 328 329 networks (here the Python library pysheds) can be applied on a virtual topography built as 330 the sum of the shortest distance to the network skeleton and the distance to the mouth along the network skeleton. For every point along the network skeleton, we can then compute the 331 332 watershed area and the upstream mainstream length, defined as the longest upstream 333 channel within the corresponding watershed.

334 Cross-sectional dimensions of tidal channels are traditionally related to the spring tidal prism (D'Alpaos et al., 2010). For tidal marsh channels, however, overmarsh tides that 335 overtop the intertidal platform are more relevant (Vandenbruwaene et al., 2013, 2015) 336 337 because maximum channel flow velocities typically occur when the surrounding platform is 338 flooded and drained (French and Stoddart, 1992). Here we use the mean overmarsh tidal 339 prism, defined as the mean tidal prism from all overmarsh tides. For every point along the 340 network skeleton, we compute the mean platform elevation of the corresponding watershed. 341 The mean overmarsh tidal prism is then simply the product between the watershed area and the mean overmarsh high-water depth, obtained from all simulated high tides higher than 342 the mean platform elevation. 343

We generate channel cross-sections along the network skeleton by balancing two constraints: cross-sections must be as perpendicular as possible to the network skeleton and consecutive cross-sections must not intersect each other. Where both constraints can be met, we then compute the channel depth as the difference between the mean channel edge elevation and the lowest cross-section elevation, the channel width as the distance between channel edges, and the cross-section area as the integral of the difference between the mean channel edge elevation and the cross-section elevation.

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